

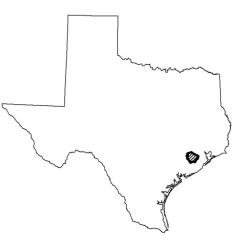
# FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

Community		
Name		
ARCOLA, CITY OF		
FAIRCHILDS, VILLAGE OF		
FIRST COLONY L.I.D.		
FORT BEND COUNTY L.I.D. NO. 2		
FORT BEND COUNTY L.I.D. NO. 7		
FORT BEND COUNTY M.U.D. NO. 2		
FORT BEND COUNTY M.U.D. NO. 23		
FORT BEND COUNTY M.U.D. NO. 25		
FORT BEND COUNTY M.U.D. NO. 34		
FORT BEND COUNTY M.U.D. NO. 35		
FORT BEND COUNTY M.U.D. NO. 41		
FORT BEND COUNTY M.U.D. NO. 42		
FORT BEND COUNTY		
UNINCORPORATED AREAS		
FULSHEAR, CITY OF		
HOUSTON, CITY OF		
KATY, CITY OF		
KENDLETON, CITY OF		
KINGSBRIDGE M.U.D.		
MISSOURI CITY, CITY OF		
NEEDVILLE, CITY OF		
PEARLAND, CITY OF		
PECAN GROVE M.U.D.		
PLEAK, VILLAGE OF		
RICHMOND, CITY OF		
ROSENBERG, CITY OF		
SIMONTON, CITY OF		
STAFFORD, CITY OF		
SUGAR LAND, CITY OF		
THOMPSONS, TOWN OF WESTON LAKES, CITY OF		
WESTON LAKES, CITY OF WEST KEEGANS BAYOU I.D.		
WILLOW FORK DRAINAGE DISTRICT		
WILLOW I ORK DRAINAGE DISTRICT		

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Community Number



#### COMMUNITIES WITH NO SPECIAL FLOOD HAZARD AREAS IDENTIFIED

Community Name	Community Number
BEASLEY, CITY OF	481654
BIG OAKS M.U.D.	481596
CHELFORD CITY M.U.D.	481568
FORT BEND COUNTY M.U.D. NO. 30	481601
MEADOWS PLACE, CITY OF	481563
MISSION BEND M.U.D. NO. 1	481578
ORCHARD, CITY OF	481655
FORT BEND COUNTY M.U.D. NO. 30 MEADOWS PLACE, CITY OF MISSION BEND M.U.D. NO. 1	481601 481563 481578

Revised: April 2, 2014



Federal Emergency Management Agency FLOOD INSURANCE STUDY NUMBER 48157CV000A

### NOTICE TO FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study may not contain all data available within the repository. It is advisable to contact the community repository for any additional data.

Part or all of this Flood Insurance Study may be revised and republished at any time. In addition, part of this Flood Insurance Study may be revised by the Letter of Map Revision process, which does not involve republication or redistribution of the Flood Insurance Study. It is, therefore, the responsibility of the user to consult with community officials and to check the community repository to obtain the most current Flood Insurance Study components.

Initial Countywide Flood Insurance Study Date Effective Date: September 30, 1992

First Revised Countywide Flood Insurance Study Date: January 3, 1997

Second Revised Countywide Flood Insurance Study Date: April 20, 2000

Third Revised Countywide Flood Insurance Study Date: November 7, 2001

Fourth Revised Countywide Flood Insurance Study Date: April 2, 2014

### TABLE OF CONTENTS

### **Page**

1.0	INTR	<b>ODUCTION</b>
	1.1 1.2 1.3	Purpose of Study       1         Authority and Acknowledgments       1         Coordination       4
2.0	AREA	<u>A STUDIED</u>
	2.1 2.2 2.3 2.4	Scope of Study4Community Description9Principal Flood Problems10Flood Protection Measures10
3.0	<u>ENGI</u>	NEERING METHODS
	3.1 3.1.1 3.1.2 3.1.3 3.2 3.2.1 3.2.2 3.2.3 3.3 3.4	Hydrologic Analyses.13New Detailed Study Streams13Redelineated Detailed Study Streams14Enhanced Approximate Study Streams24Hydraulic Analyses25New Detailed Study Streams25Redelineated Detailed Study Streams26Enhanced Approximate Study Streams26Enhanced Approximate Study Streams29Land Subsidence29
4.0	FLOC	DDPLAIN MANAGEMENT APPLICATION
	4.1 4.2	Floodplain Boundaries
5.0	INSU	RANCE APPLICATION
6.0	<u>FLOC</u>	DD INSURANCE RATE MAP
7.0	<u>OTHE</u>	E <u>R STUDIES</u>
8.0	LOCA	ATION OF DATA
9.0	<b>BIBL</b>	IOGRAPHY AND REFERENCES

## TABLE OF CONTENTS (Cont'd)

### **FIGURES**

Figure 1 – NGS-HGCSD Benchmarks	31
Figure 2 – Fort Bend Subsidence Adjustment Values from 1973 to 2000	31
Figure 3 – Land Subsidence Schematic	33
Figure 4 – Floodway Schematic	35
Figure 4 – Floodway Schematic	35

### **TABLES**

Table 1 – Scope of Study	5
Table 2 – Letters of Map Revision	7
Table 3 – Levees in the Brazos River	
Table 4 – Redelineated Detailed Study Streams	
Table 5 – Summary of Discharges	
Table 6 – Summary of Stillwater Elevations	
Table 7 – Enhanced Approximate Study Streams	
Table 8 - Subsidence Adjustment Values for Redelineated Streams	
Table 9 – Manning's "n" Values	
Table 10 – Floodway Data Table	
Table 11 – Community Map History	60

### **EXHIBITS**

Exhibit 1 – Flood Profiles

Alcorn Bayou	Panel	01P
Brazos River	Panels	02P-20P
Cane Island Branch	Panel	21P
Clear Creek	Panel	22P
Clodine Ditch/Long Point Slough	Panel	23P
Coon Creek	Panels	24P-26P
Cow Creek	Panels	27P-28P
Dry Creek	Panels	29P-33P
Keegans Bayou	Panels	34P-35P
Little Prong Buffalo Bayou - Panel Removed	Panel	36P
Little Prong Buffalo Bayou Side Channel	Panel	37P
Long Point Creek	Panel	38P
Long Point Creek East Fork	Panel	39P
Lower Oyster Creek	Panels	40P-41P
Mustang Bayou	Panels	42P-43P
Mustang Bayou Diversion Channel	Panel	44P
North Branch Dry Creek	Panel	45P

### **Page**

Oyster Creek	Panels	46P-52P
Red Gully	Panels	53P-54P
San Bernard River	Panels	55P-56P
Seabourne Creek	Panels	57P-59P
Snake Creek	Panel	60P
Snake Slough	Panel	61P
Stafford Run	Panels	62P-63P
Steep Bank Creek	Panel	64P
Tributary 20.25 to Sims Bayou	Panel	64 (a)P
Willow Fork Buffalo Bayou	Panels	65P-67P
Willow Fork Diversion Channel	Panel	68P

Exhibit 2 – Flood Insurance Rate Map Index Flood Insurance Rate Maps

#### FLOOD INSURANCE STUDY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

#### 1.0 INTRODUCTION

#### 1.1 Purpose of Study

This Flood Insurance Study (FIS) revises and updates information on the existence and severity of flood hazards in the geographic area of Fort Bend County, Texas including Big Oaks M.U.D.; Chelford City M.U.D, the First Colony Levee Improvement District (L.I.D.); Fort Bend County L.I.D. Nos. 2 and 7; Fort Bend County Municipal Utility District (M.U.D) Nos. 2, 23, 25, 30, 34, 35, 41, and 42; Kingsbridge M.U.D.; Pecan Grove M.U.D.; West Keegans Bayou Improvement District (I.D.); Willow Fork Drainage District; Cities of Arcola, Beasley, Fulshear, Houston (within Fort Bend County), Katy (within Fort Bend County), Kendleton, Meadows Place, Mission Bend M.U.D. No.1, Missouri City (within Fort Bend County), Needville, Orchard, Pearland (within Fort Bend County), Richmond, Rosenberg, Simonton, Stafford, Sugar Land, Weston Lakes; the Town of Thompsons; the Villages of Fairchilds and Pleak; and the unincorporated areas of Fort Bend County (referred to collectively herein as Fort Bend County). The following jurisdictions are considered non-flood prone for their areas within Fort Bend County: the Big Oaks M.U.D.; Chelford City M.U.D.; Fort Bend County M.U.D. No. 30; Mission Bend M.U.D. No.1; and Cities of Beasley, Meadows Place, and Orchard. The Cities of Houston, Katy, Missouri City, and Pearland are located in two or more counties. Also, Fort Bend County M.U.D. No. 81 has been annexed by the City of Weston Lakes and no longer exists.

This FIS aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This study has developed flood-risk data for various areas of the community that will be used to establish actuarial flood insurance rates and to assist the community in its efforts to promote sound floodplain management. Minimum floodplain management requirements for participation in the NFIP are set forth in the Code of Federal Regulations at 44 CFR, 60.3.

In some States or communities, floodplain management criteria or regulations may exist that are more restrictive or comprehensive than the minimum Federal requirements. In such cases, the more restrictive criteria take precedence, and the State (or other jurisdictional agency) will be able to explain them.

#### 1.2 Authority and Acknowledgments

The sources of authority for this FIS report are the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

This revision of the FIS was prepared by the Comprehensive Flood Risk Resources & Response Joint Venture (hereinafter referred to as CF3R) for FEMA under contract No. EMT-2002-CO-0049. This effort was part of the Digital FIRM (DFIRM) Update for Fort Bend County which was a joint effort by the Department of Homeland Security's Federal

Emergency Management Agency (FEMA) and its Cooperating Technical Partners (CTP), Fort Bend County and the City of Sugar Land.

Information concerning the authority and acknowledgements for each jurisdiction shown on this countywide FIS, compiled from the 2001 FIS report, is detailed below.

#### Fort Bend County and Certain Political Districts

The hydrologic and hydraulic analyses in the original study were prepared by Espey, Huston & Associates, Inc., now Atkins for the Department of Home Land Security's FEMA, under Contract No. H-4569. The work for that study was completed in April 1980. Also included are the hydrologic and hydraulic analyses for the Brazos River, which were prepared by Atkins. This work was completed in May 1985. In the June 3, 1988 revision, updated hydrologic and hydraulic analyses for portions of Dry Creek were prepared by LJA Engineering & Surveying, Inc. The work for that revision was completed in April 1987. In the May 3, 1990 revision, updated hydrologic and hydraulic analyses for Clodine Ditch were prepared by Vansickle, Mickelson & Klein, Inc. The work for that revision was completed in October 1988. In the June 18, 1990 revision, updated hydrologic and hydraulic analyses for Red Gully were prepared by LJA Engineering & Surveying, Inc. The work for that revision, updated hydrologic and hydraulic analyses for Red Gully were prepared by LJA

#### First Colony L.I.D., and the City of Fulshear

The hydrologic and hydraulic analyses for these studies were prepared by Atkins, during the preparation of the FIS for Fort Bend County. That study was prepared for FEMA, under Contract No. H-4569. The work for that study was completed in May 1985.

*Fort Bend County L.I.D. No.2, Fort Bend County M.U.D. No.2, and Kingsbridge M.U.D.* The hydrologic and hydraulic analyses for these studies were prepared by Atkins.

#### Fort Bend County M.U.D. No. 25

The hydrologic and hydraulic analyses for this study were prepared by Atkins. Updated hydraulic analyses for Red Gully were prepared by Dewberry & Davis in July 1985.

#### Pecan Grove M.U.D.

The hydrologic and hydraulic analyses for this study were prepared by Atkins in November 1985.

#### City of Missouri City

The hydrologic and hydraulic analyses for the original study were prepared by Atkins, for FEMA, under Contract No. H-4569. That work was completed in April 1980. Updated hydrologic and hydraulic analyses for the Brazos River were prepared by Atkins. This work was completed in May 1985.

#### City of Richmond

The hydrologic and hydraulic analyses for the original study were prepared by Atkins for FEMA, under Contract No. H-4569. That work was completed in April 1980. Updated hydrologic and hydraulic analyses for the Brazos River were prepared by Atkins. That work was completed in May 1985. A revision was also prepared by Dewberry & Davis to reflect updated corporate limits for the city. That work was completed in August 1986.

#### City of Rosenberg

The hydrologic and hydraulic analyses in the original study were prepared by Atkins for FEMA, under Contract No. H-4569. That work was completed in April 1980. Updated hydrologic and hydraulic analyses for Dry Creek and North Branch Dry Creek were prepared by Vansickle, Mickelson & Klein, Inc., under agreement with FEMA. That work was completed in October 1988. Updated hydrologic and hydraulic analyses for portions of Seabourne Creek were prepared by LJA Engineering & Surveying, Inc, for FEMA. That work was completed in December 1988. Updated hydrologic and hydraulic analyses for the Brazos River were prepared by Atkins for FEMA, during the preparation of the FIS for Fort Bend County. That work was completed in December 1988.

#### City of Simonton

The hydrologic and hydraulic analyses for this study were prepared by Atkins for FEMA, during the preparation of the Flood Insurance Study for Fort Bend County. That work was completed in May 1985.

#### City of Stafford

The hydrologic and hydraulic analyses for this study were performed by Atkins for FEMA, under Contract No. H-4569. That work was completed in April 1980.

#### City of Sugar Land

The hydrologic and hydraulic analyses for the original study were prepared by Atkins for FEMA, under Contract No. H-4569. The work for that study was completed in April 1980. Updated hydrologic and hydraulic analyses were prepared by Atkins. That work was completed in May 1985. A revision was also prepared by Dewberry & Davis to reflect updated corporate limits for the city. That work was completed in June 1986.

#### Village of Pleak

The hydrologic and hydraulic analyses for this study were prepared by Atkins during the preparation of the FIS for Fort Bend County. The work for that study was completed in April 1980.

The effects of updated hydrologic and hydraulic analyses for Dry Creek, incorporated in the City of Rosenberg FIS, were prepared by Vansickle, Mickelson, & Klein, Inc. That work was completed in October 1988.

For this revision, the hydrologic and hydraulics analyses for Oyster Creek, Lower Oyster Creek, and Brazos River were prepared by CF3R. The hydrologic and hydraulics analyses for Cane Island, Clear Creek (from the Fort Bend/Harris County line to FM 2234), and Keegans Bayou (from Fort Bend/Harris County line to Belknap Road) were prepared as part of the Harris County and Incorporated Areas Flood Insurance Study, dated June 18, 2007 (Harris County 2007 FIS) (Reference 1). In addition, the flood hazard boundaries for other detailed study streams were redelineated based on the 2001 floodplain data, approved LOMR's, and the LiDAR topographic data collected in 2005. The 2001 flood hazard boundaries for zone A streams were remapped based on enhanced approximate studies. All the work was completed in September 2009.

#### 1.3 Coordination

The dates of the initial and final Consultation and Coordination Officer's (CCO) meetings held for Fort Bend County and the incorporated communities within its boundaries are shown in the following tabulation:

Community Name	Initial CCO Date	Final CCO Date
Fort Bend County and		
Certain Political Districts	May 12, 1978	June 11, 1984
First Colony L.I.D.	*	September 26, 1986
Fort Bend County L.I.D. No. 2	*	November 17, 1983
Fort Bend County M.U.D. No. 2	*	November 17, 1983
Fort Bend County M.U.D. No. 25	*	November 21, 1985
Fort Bend County M.U.D. No. 81		
Kingsbridge M.U.D.	*	November 17, 1983
Mission Bend M.U.D. No. 1	*	*
Pecan Grove M.U.D.	*	September 26, 1986
City of Fulshear	*	September 26, 1986
City of Houston	*	*
City of Missouri City	May 12, 1978	April 4, 1981
City of Needville	*	*
City of Richmond	May 12, 1978	April 10, 1981
City of Rosenberg	May 12, 1978	April 10, 1981
City of Simonton	*	September 25, 1986
City of Stafford	May 12, 1978	April 9, 1981
City of Sugar Land	May 12, 1978	April 9, 1981
Village of Pleak	*	*

\* Unavailable

For this revision, the initial CCO meeting was held on May 10, 2004 and attended by representatives of FEMA, CF3R, community and county officials, other interested agencies and citizens.

The results of the study were reviewed at the final CCO meeting held on January 26, 2010, and attended by representatives of FEMA, the State of Texas, the communities, and the study contractor. All problems raised at that meeting have been addressed in this study.

#### 2.0 AREA STUDIED

#### 2.1 Scope of Study

This FIS covers the geographic area of Fort Bend County, Texas, including the incorporated communities listed in Section 1.1. Table 1, "Scope of Study", lists the limits of detailed study for flooding sources studied by the detailed and redelineation methods in this revision.

### TABLE 1 – SCOPE OF STUDY

Detailed Study Streams	Limits of Detailed Study
Brazos River	For its entire length within the county
Cane Island Branch	From the confluence with Willow Fork Buffalo Bayou to the County Boundary.
Clear Creek	From the upstream of the Farm Road 34 to the downstream side of the upper FM 2234 crossing
Keegans Bayou	From the County Boundary to the downstream of Belknap Road
Oyster Creek	From Jones Creek to Flat Bank diversion channel in Missouri City
Lower Oyster Creek	From Flat Bank diversion channel to the Sienna Plantation Levee diversion channel
Redelineated Detailed Study Streams	Limits of Detailed Study
Clear Creek	From the upstream side of the upper FM 2234 crossing to upstream of the Roven Road
Clodine Ditch/Long Point Slough	From the downstream county boundary to a point approximately 3.15 miles upstream of Harlem Road
Coon Creek	From a point approximately 130 feet downstream of Band Road to Randon Road
Cow Creek	From its confluence with the Brazos River to a point approximately 8.14 miles upstream
Dry Creek	From the downstream face of Berdett Road to a point approximately 1,100 feet upstream of State Route 36
Keegans Bayou	From the upstream of Belknap Road to a point approximately 0.9 mile upstream of Gaines Road
Little Prong Buffalo Bayou	From its confluence with Willow Fork Buffalo Bayou to a point approximately 2.5 miles upstream

#### TABLE 1 - SCOPE OF STUDY - Continued

Redelineated Detailed Study Streams	Limits of Detailed Study
Little Prong Buffalo Bayou Side Channel	From its confluence with Willow Fork Buffalo Bayou to a point approximately 1.3 miles upstream
Long Point Creek	From Briscoe Canal to a point approximately 0.5 mile upstream of Trammel-Fresno Road
Long Point Creek East Fork	From its confluence with Long Point Creek to a point approximately 0.7 mile upstream of Trammel-Fresno Road
Mustang Bayou	From the downstream county boundary to a point approximately 1,600 feet upstream of Turtle Creek Drive
North Branch Dry Creek	From its confluence with Dry Creek to a point approximately 470 feet upstream of Leonard Street
Red Gully	From its confluence with Oyster Creek to FM 1464
Seabourne Creek	From a point approximately 0.4 mile downstream of State Route 36 to the upstream side of Scott Road
Stafford Run	From a point approximately 1,030 feet upstream of its confluence with Oyster Creek to a point approximately 0.3 mile upstream of Brand Road
Willow Fork Buffalo Bayou	For its entire length within Fort Bend County
Willow Fork Diversion Channel	From the City of Houston corporate limits to its divergence from Willow Fork Buffalo Bayou

Base flood elevations, 1% and 0.2% annual chance floodplain boundaries, and floodway delineations for Dry Creek were revised from a point approximately 4,000 feet downstream of Bryan Road to Bryan Road. These changes resulted from the effects of updated hydrologic and hydraulic analyses, performed by Vansickle, Mickelson, & Klein, Inc., for Dry Creek in the FIS for the City of Rosenberg (Reference 2).

This revision incorporates the effects of annexations or deannexations of most communities in Fort Bend County. In addition, the corporate limits for the City of Weston Lakes and the City of Pearland have been added to the FIRM for this revision.

This revision also incorporates Letters of Map Revision that were issued by FEMA after the latest date, June 27, 1996, shown on Table 1 of the 2001 FIS report. Note that some were only partially incorporated as portions were subsequently revised by a later LOMR. They are listed in Table 2, "Letters of Map Revision."

### TABLE 2: LETTERS OF MAP REVISION

Project	Stream	Date
Fort Bend County L.I.D. No. 2	Ditch A, Ditch C, Ditch E, Lakes of Edgewater, Grants Lake	October 16, 1997
Bulkhead Lake	Oyster Creek	January 7, 1998
Fort Bend County L.I.D. No. 10	Brazos River	April 22, 1998
Fort Bend County L.I.D. No. 14	Brazos River	May 8, 1998
Fort Bend County L.I.D. No. 2	Ditch A	October 19, 1998
First Colony L.I.D.	Oyster Creek, Unnamed Tributary to Oyster Creek, Two Unnamed Ponds	December 29, 1998
First Colony L.I.D.	Oyster Creek	September 30,1999
Fort Bend County L.I.D. No. 11	Brazos River	November 5, 1999
Willow Fork Buffalo Bayou, Katy- Flewellen Road to Downstream of FM 1463	Willow Fork Buffalo Bayou	January 31, 2000
West Keegans Bayou Improvement District	Keegans Bayou	February 10, 2000
Fort Bend County M.U.D. Nos. 34 and 35	Little Prong Buffalo Bayou, Little Prong Buffalo Bayou Side Channel	March 31, 2000
Stafford Run Improvement	Stafford Run	March 23, 2001
First Colony L.I.D., Colony Bay L.I.D., Riverstone Development	Interior Levee Drainage	June 12, 2002
FM 1463 Katy Tract	Willow Fork Buffalo Bayou	August 20, 2002
Sienna Plantation L.I.D.	Brazos River	August 22, 2002
Sprint Landfill	Red Gully	December 31, 2002
Seabourne Creek Improvement	Seabourne Creek	October 29, 2003
First Colony L.I.D., Colony Bay L.I.D., Riverstone Development	Interior Levee Drainage	November 12, 2003
Firethorne Property	Willow Fork Buffalo Bayou	January 14, 2004
Fort Bend L.I.D No. 14	Brazos River	February 10, 2004
Fort Bend County M.U.D. No.121	Brazos River	March 15, 2004

### TABLE 2: LETTERS OF MAP REVISION - Continued

Project	Stream	Date
Fort Bend County MUD 23	Long Point Creek	April 30, 2004
The Crossings at Riverstone Phase 3	Unnamed Zone A Ponding Area	August 17, 2004
The Crossings at Riverstone Phase 3	Unnamed Zone A Ponding Area	September 27, 2004
Olympia Estates	Mustang Bayou	December 9, 2004
Firethorne Property	Willow Fork Buffalo Bayou	January 14, 2005
Olympia Estates (map correction)	Mustang Bayou	January 27, 2005
Katy Creek Ranch Development	Willow Fork Buffalo Bayou	August 10, 2005
Sienna North Levee	Brazos River	May 26, 2006
Fort Bend L.I.D. No 17	Brazos River	January 25, 2007
Woodcreek Reserve	Willow Fork Buffalo Bayou	February 26, 2007
Seabourne Trace	Seabourne Creek	May 31, 2007
Winfield Lakes	Long Point Creek	September 27, 2007
Woodcreek Commercial Reserve	Willow Fork Buffalo Bayou	December 14, 2007
Fort Bend County MUD 23	Long Point Creek East Fork	December 31, 2008
Fort Bend L.I.D. No.10	Brazos River	May 13, 2009
Woodcreek Reserve	Willow Fork Buffalo Bayou	August 17, 2009
Fort Bend L.I.D. No 17	Brazos River	August 21, 2009
Fort Bend L.I.D. No.6	Brazos River	January 5, 2010
West WWTP	Brazos River	March 19, 2010
Fort Bend L.I.D. No 15	Brazos River; Alcorn Bayou; Snake Slough; Steep Bank Creek	April 2, 2010
Fort Bend L.I.D. No 19	Brazos River; Steep Bank Creek; Lake 83	April 9, 2010
Fort Bend L.I.D. No.20	Brazos River; Lake 1	March 30, 2011
Lower Snake Creek	Snake Creek	July 8, 2011
Fort Bend LIDs No. 15 and 19	Alcorn Bayou, Snake Slough, SB Lakes 81 & 82, Diversion	July 5, 2012

#### TABLE 2: LETTERS OF MAP REVISION - Continued

Project	Stream	Date
Fort Bend LID 15 Internal System	Alcorn Bayou, Snake Slough, AB Lakes A1 & A2, SS Lake 1	June 6, 2013
Katy Main Street	Willow Fork Buffalo Bayou & Cane Island Branch	August 2, 2013

The San Bernard River, Turkey Creek, Snake Creek, Cottonwood Creek, Buffalo Creek, Fairchilds Creek, Big Creek, Deer Creek, Cedar Creek, Bessies Creek, Brookshire Creek, Bullhead Bayou, Jones Creek, Rabbs Bayou, and the remaining portions of Coon Creek, Seabourne Creek, Clear Creek, Dry Creek, Long Point Creek, Oyster Creek, Red Gully, Cow Bayou, Flat Bank Bayou, Albine Lake, Brooks Lake, Clear Lake, Eldridge Lake, Horseshoe Lake, North Lake Sugar Creek, South Lake Sugar Creek, North Pecan Lake, Lake Jane Long, Cleveland Lake, Brooks Lake Diversion Channel, Ditch H, Venetian Lake, Ditch A, Ditch B, Ditch B-1, Ditch C, Ditch C-l, Ditch E, Ditch F, Brooks Lake, Omar Lake Diversion, Flewellan Creek, Mound Creek, Cedar Creek, San Bernard River, Sugar House Lake, Lawson Lake, Brushy Lake, Foss Creek, Smithers Lake, an unnamed tributary to Oyster Creek, and an unnamed lake were studied by approximate methods. Portions of Stafford Run were also studied by approximate methods. Approximate analyses were used to study those areas having a low development potential or minimal flood hazards. The scope and methods of study were proposed to, and agreed upon by FEMA and communities in Fort Bend County.

#### 2.2 Community Description

Fort Bend County is located along the Gulf Coast region in the US state of Texas within the Houston-Sugar Land-Baytown metropolitan area. It is bordered by Waller County to the north, Wharton County to the southwest, Harris County to the east, Brazoria County to the southeast, and Austin County to the northwest. In 2000 the Fort Bend County's population was 354,452; in 2008, the US Census Bureau estimated its population to have reached 532,141, a 50% growth rate in eight years from the last US Census. Since the 1970s Fort Bend County has been one of the fastest-growing counties in the United States. Its county seat is Richmond, while its largest city is Sugar Land (Reference 3).

The county comprises 869 square miles of level to slightly rolling terrain with an elevation ranging from eighty to 250 feet above sea level. Temperatures range from an average high of 94° F in July to an average low of 44° F in January; rainfall averages slightly more than forty-five inches a year, and the growing season lasts 296 days. The Brazos River flows diagonally northwest to southeast through the county and drains the broad central valley via numerous creeks and bayous. The San Bernard River, which forms the west boundary, drains the western quarter of the county. Major streams include Big Creek, which flows east into the Brazos River; Oyster Creek, which winds parallel to and east of the Brazos River; and Buffalo Bayou, which rises in the northern tip of the county and flows east into Harris County. Soils vary from rich alluvial in the Brazos

valley to sandy loams and clay on the prairies. Native trees include pecan, oak, ash, and cottonwood; there are some timberlands in the north and along streams. Mineral resources include natural gas, oil, and sulfur; sand, clay, and gravel are also produced in commercial quantities (Reference 4).

#### 2.3 Principal Flood Problems

Reports on historic flooding show that major storms or floods in the area occurred in 1899, 1900, 1913, 1915, and 1929. The flood of 1899 and the storm of 1900 caused much damage to Fort Bend County. Crops, stock, and lives were lost during these two events. The City of Richmond suffered some of the greater losses. Four to five feet of water was seen for several days, and in some places for a stretch of land seven miles wide. The floods of 1913 and 1929 left water covering large portions of the Cities of Rosenberg and Richmond. It was reported that during the 1913 flood, the waters of the Brazos, San Bernard, and Colorado Rivers met below Rosenberg.

The 1991 flooding event caused by heavy rains from the Gulf of Mexico storm was one of the largest rainfall totals in Texas recorded history. Downstream, the Brazos River and Oyster Creek merged as the Brazos River flowed over the left floodplain near Harris Reservoir. Residential flooding was widespread above in Simonton to the Gulf in Fort Bend and Brazoria Counties (Reference 5).

The October 1994 floods were the deadliest Southeast Texas weather event since 1983's Hurricane Alicia. Heavy rains began falling late afternoon on the 16th across Burleson, Brazos, Grimes and Washington counties. On the night of the 17th and on the 18th rains continued to slide further south and began affection people in Jackson, Wharton, Matagorda, Brazoria, and portions of Fort Bend counties. Total rainfall from the entire storm generally ranged from 10 to 20 inches with Liberty recording 30.50 inches during the storm. Over 13,000 people had to be evacuated during the floods and over 22,000 homes received flood damage. Total damage to homes and businesses was approximately \$800 million while another \$100 million was done to roads and bridges throughout Southeast Texas (Reference 6).

The October 1998 Texas flooding was a flood event that occurred across parts of South Texas and Southeast Texas. The storm that caused it was one of the costliest in the recorded meteorological history of the United States, bringing rainfall of over 20 inches to some parts of Southeast Texas (the Houston-Sugar land-Baytown and Beaumont – Port Arthur metropolitan areas) and causing over \$750 million in damages (Reference 7).

#### 2.4 Flood Protection Measures

Diversions from Oyster Creek to the Brazos River by way of the Brooks Lake, which is located near the intersection of U.S. Highway 59 and State Highway 6, and to Jones Creek north of Richmond has reduced flooding along Oyster Creek.

Keegans Bayou was channelized in 1984 by The West Keegans Bayou Improvement District (WKBID) (Reference 8). In 1999, WKBID constructed detention ponds along

Keegans Bayou at upstream and downstream of Gaines Road. The channelization and detention ponds provide protection against the 1% annual chance flood from Keegans Bayou in Fort Bend County.

A channelization project has been completed on Red Gully by the U. S. Home Corporation. This project included the channel deepening and widening of Red Gully from the confluence with Oyster Creek to a point approximately 7,500 feet upstream within the corporate limits of Fort Bend County M.U.D. No. 25. The channelization was designed to contain the 1% annual chance flood elevation of Red Gully. A diversion channel has been constructed from the confluence with Mustang Bayou to Hermann Hospital Lake (in Missouri City) in 2004 to reduce the 1% flooding of the Mustang Bayou. A channel improvement along Stafford Run and construction of four in-line detention ponds from Northpark Drive to Brand Road has been completed in 2000. The project has reduced the 1% flood elevations of Stafford Run.

A large percentage of development in Fort Bend County (FBC) has occurred along the Brazos River within the 1% annual chance floodplain. These developments are protected by levees constructed and maintained by several L.I.D.s and M.U.D.s. Due to the revised base flood elevations of the Brazos River study from this revision and FEMA reemphasis on levee recertification through Procedure Memorandums 32 and 34 (References 9 and 10), FBC has spearheaded a parallel program to raise the levee systems to bring them into compliance with FEMA certification requirements. Several of the levee systems are interconnected, with the outer perimeter levees providing primary protection and the interior levees providing secondary layers of protection.

On the north side of the Brazos River, FBC L.I.D.s 2, 14, 15, 19, First Colony L.I.D. and L.I.D. 2, and FBC M.U.D. 46 cooperated on raising the perimeter system which connects their districts. On the south side of the River, FBC L.I.D.s 10, 11 and M.U.D. 121 along with the newly created FBC L.I.D. 6, cooperated on raising the existing levees and building two new segments of levee to connect these districts with a perimeter system. The remaining districts (FBC L.I.D. 17 and L.I.D. 7) undertook independent projects to raise their respective levees. Except for FBC L.I.D. 7, those projects have been completed and re-certifications have been submitted to FEMA for processing. Improvement to the Pecan Grove Levee is under design at the completion of this revision. All constructed levees along the Brazos River from U.S. 90A to the South Texas Water Company Canal are listed in Table 3.

#### TABLE 3: LEVEES IN THE BRAZOS RIVER

Note

#### Levees

Pecan Grove M.U.D. Included in the 2001 FIS report Included in the 2001 FIS report Fort Bend County L.I.D. 7 Included in the 2001 FIS report Fort Bend County L.I.D. 10 Included in the 2001 FIS report Fort Bend County L.I.D. 2 Included in the 2001 FIS report First Colony L.I.D. Included in the 2001 FIS report First Colony L.I.D. 2 Included in the 2001 FIS report Fort Bend County M.U.D. 46 Included in the 2001 FIS report Fort Bend County M.U.D. 49 Included in the 2001 FIS report Sienna Plantation L.I.D. Fort Bend County L.I.D. 14 LOMR Case #98-06-784P Fort Bend County L.I.D. 11 LOMR Case #99-06-1722P North Sienna Plantation L.I.D. LOMR Case #02-06-266P Fort Bend County M.U.D. 121 LOMR Case #03-06-449P Fort Bend County L.I.D. 17 LOMR Case #06-06-BD92P Fort Bend County L.I.D. 15 LOMR Case #07-06-2682P Fort Bend County L.I.D. 6 LOMR Case #09-06-2928P Fort Bend County L.I.D. 19 LOMR Case #09-06-0987P LOMR Case #11-06-1803P Fort Bend County L.I.D. 20

There are also levees to protect the flooding from Oyster Creek in Fort Bend L.I.D. No.2, First Colony L.I.D., M.U.D. 46, and M.U.D. 49. The levees in Fort Bend County that meet the Code of Federal Regulations at 44 CFR, 65.10 are shown on the DFIRMs with the appropriate notes.

#### 3.0 ENGINEERING METHODS

For the flooding sources studied in detail in the community, standard hydrologic and hydraulic study methods were used to determine the flood-hazard data required for this study. Flood events of a magnitude that is expected to be equaled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for floodplain management and for flood insurance rates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10-, 2-, 1-, and 0.2-percent annual chance, respectively; of being equaled or exceeded during any year. Although the recurrence interval represents the long-term, <u>average</u> period between floods of a specific magnitude, rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood, which equals or exceeds the 1-percent-annual-chance flood in any 50-year period is approximately 40 percent (4 in 10); for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported herein reflect flooding potentials based on conditions existing in the community at the time of completion of this study. Maps and flood elevations will

be amended periodically to reflect future changes.

#### 3.1 Hydrologic Analyses

Hydrologic analyses were performed to establish peak discharge-frequency relationships for each flooding source studied by detailed methods affecting the community.

#### 3.1.1 New Detailed Study Streams

Six new detailed studies of Cane Island, Clear Creek, Keegans Bayou, Oyster Creek, Lower Oyster Creek, and Brazos River were completed in this revision.

For Cane Island Branch, hydrology analysis was developed as part of the Harris County 2007 FIS, using the US Army Corps of Engineers' (USACE) HEC-HMS computer program (Reference 11).

For Clear Creek, from FM 2234 to the Harris and Fort Bend County line, hydrology analysis was developed as part of the Harris County 2007 FIS, using the USACE HEC-HMS computer program.

For Keegan Bayou, from Belknap Road to the Harris and Fort Bend County line, hydrology analysis was developed as part of the Harris County 2007 FIS, using the USACE HEC-HMS computer program.

The study for Tributary 20.25 to Sims Bayou from the upstream side of Highway 8 to just downstream of Maywood St. was taken from the Harris County 2007 FIS.

The 10-, 2-, 1-, 0.2-percent annual chance discharges for the Oyster Creek and Lower Oyster Creek were determined using the USACE HEC-HMS computer program. Rainfall data were obtained from the Fort Bend Drainage Criteria Manual (FBDCM) dated 1999 (Reference 12). No aerial adjustments were made to the point rainfall data. Drainage areas were delineated based on LiDAR topography data collected in 2005. Land use data was developed based on the County GIS data and 2005 aerial imagery. The Green-Ampt loss methodology was used to compute infiltration loss. Clark Unit Hydrograph was used to calculate runoff volume with the time of concentration,  $T_{c}$ , and Storage Coefficient *R* computed using the methodology from the FBDCM. The modified Puls Routing method was used to route hydrographs between model nodes.

The 10-, 2-, 1-, 0.2-percent annual chance discharges for Brazos River was determined by performing a flood frequency analysis on a modified set of the Richmond Gage data in accordance with Bulletin 17B (Reference 13). Frequency analysis assumes a stationary data sequence. Construction of the upstream reservoirs has introduced non-stationary data. Bulletin 17B does not provide guidance when watershed changes have affected the magnitude, homogeneity, or randomness of measured peak discharges. The annual peak discharge data from the USGS Gage at Richmond (Station 0811400) from 1923 to 2004 together with selected historic flood data were utilized as the main component of the flood frequency analysis. Based on the reservoir stage-storage data, the total runoff volume for each event was estimated and relationship was developed between the

"regulated" and "unregulated" flows. Once the recorded flows were converted to "unregulated" flows, flood frequency analysis for the unregulated flows was applied to determine "unregulated" peak flows for different return intervals. The conversion factors used in the prior effective FIS report (dated November 2001) were utilized to convert the "unregulated" flows into "fully regulated" conditions (reference 14).

#### 3.1.2 Redelineated Detailed Study Streams

There is no new hydrology study for the redelineated detailed study streams. The hydrology data for those streams is as described in the Fort Bend County 2001 FIS report and LOMR's documents. Redelineated streams are listed in Table 4.

<u>Streams</u>	FIRM Panel #	Stream Profile #
Clear Creek Clodine Ditch/Long Point Slough	48157C0305L, 48157C0310L 48157C0110L, 48157C0130L	20P 21P
Coon Creek	48157C0225L, 48157C0240L, 48157C0400L	22P, 23P, 24P
Cow Creek	48157C0500 L, 48157C0575L	25P, 26P
Dry Creek	48157C0240LJ, 48157C0245L, 48157C0265L, 48157C0270L, 48157C0425L	27P, 28P, 29P
Keegans Bayou	48157C0140L, 48157C0145L, 48157C0165L	32P, 33P
Little Prong Buffalo	48157C0105L, 48157C0110L	34P
Long Point Creek	48157C0315L, 48157C0455L	36P
Mustang Bayou	48157C0285L, 48157C0295L, 48157C0305L, 48157C0315L 48157C0270 J	40P, 41P
Red Gully	48157C0140L, 48157C0145L	51P, 52P
Seabourne Creek	48157C0240L, 48157C0245L, 48157C0400L	55P, 56P
Stafford Run	48157C0165L, 48157C0280L, 48157C0285L	57P, 58P
San Bernard River	48157C0200L, 48157C0350L, 48157C0375L, 48157C0500L, 48157C0525L	53P, 54P,
Willow Fork Buffalo Bayou	48157C0040L, 48157C0045L, 48157C0105L, 48157C0110L, 48157C0130L	60P, 61P

#### TABLE 4: REDELINEATED DETAILED STUDY STREAMS

For Clear Creek, from Rouen Road to FM 2234, the hydrologic analysis was completed using the USACE HEC-l computer program (Reference 15). The revised HEC-l analysis, dated August 1991, was included in a report entitled "Clear Creek Regional Flood Control Plan, Hydraulic Baseline Report," prepared by Dannenbaum Engineering Corporation, for the Harris County Flood Control District and Texas Water Development Board, and dated September 1991. The discharges decreased compared to the previous determined discharges as a result of the updated watershed conditions (Reference 16).

The 10-, 2- and 1-percent-annual-chance flood discharges for the lower portion of Clodine Ditch were determined from a synthetic hydrograph analysis developed by Vansickle, Mickelson & Klein, Inc., using the USACE HEC-1 computer program (References 17). The 0.2-percent-annual-chance flood discharges were extrapolated from the values for the lower-frequency floods. Discharges for the upper portion of Clodine Ditch, with a drainage area of less than 5.25 square miles, were determined using USGS Water Resources Investigations 3-73 (Reference 18).

For Keegans Bayou, from upstream of Gaines Road to Belknap Road, the hydrologic analyses were developed from a synthetic hydrograph analysis developed by Turner, Collie & Braden, Inc., using the USACE HEC-1 computer program. Updated hydrologic analysis to reflect existing watershed conditions along Keegans Bayou within the City of Houston and the unincorporated areas of Fort Bend County was completed by RUST Environment and Infrastructure. That work was completed in 2000.

For the 10- and 1-percent-annual-chance floods for Coon Creek, North Branch Dry Creek, Seabourne Creek, and Stafford Run; and the 10-, 2-, and 1-percent-annual-chance floods for Cow Creek, Mustang Bayou, Long Point Creek, and Dry Creek, the regionalized USGS methodology was used to determine the flood flows for the selected recurrence intervals (Reference 19). The 0.2-percent-annual-chance floods for each of these streams were extrapolated from lower frequency values. Drainage areas were determined from USGS topographic maps at a scale of 1:24,000 with a contour interval of 5 feet (Reference 20). For the portion of Dry Creek from the downstream face of Berdett Road to upstream of State Route 36, the 10-, 2-, 1-, and 0.2-percent-annual-chance discharges were determined using USGS Water Resources Investigation 3-73. The 0.2-percent-annual-chance flows were derived by extrapolating the 10-, 2-, and 1-percent-annual-chance discharges on log probability paper.

For Red Gully, the 10-, 2-, 1-, and 0.2-percent-annual-chance discharges were calculated using methodologies developed by the Fort Bend DCM, using the USACE HEC-1 computer program. For the portion of Red Gully in Fort Bend County M.U.D. No. 25, the 10- and 1-percent-annual-chance flows were determined using a regionalized USGS methodology, with drainage areas determined from USGS quadrangle maps. For this portion, 2- and 0.2-percent-annual-chance flows were not calculated.

For the San Bernard River from approximately 10,800 feet upstream of the Atchison, Topeka, and Santa Fe Railway to approximately 3,000 feet upstream of its confluence with Peach Creek, the 10-, 2-, 1-, and 0.2-percent exceedance probabilities were determined by the USACE, Galveston District, for the FEMA under Contract No. EMW-97-IA-0140 (Wharton County, Texas). This work was completed in September 1998.

Official records for the San Bernard River show record floods in 1960, 1973, and 1985. Peak discharges for the selected exceedance probabilities were computed using newly published regional regression equations for the State of Texas by the U.S. Geological Survey (USGS) (Reference 21). The San Bernard River watershed is in Region 11. An urbanization adjustment was not considered on the flooding source because there is currently less than 10% development within the watershed which was determined by field visits and orthographic images.

A flood frequency analysis was performed on the San Bernard River at the gage near Boling, Texas (FM 442) to compare those frequency discharges to the regional regression equations that were used for the detailed analysis. The regression equation discharges were within 3% of the frequency analysis, so the regression equations were used on the San Bernard River.

Peak discharge-drainage area relationships for all flooding sources studied in detail are shown in Table 5, "Summary of Discharges".

Flood elevations were computed by detailed methods for several ponds. These were all incorporated through the LOMR process. Table 6 provides a summary of the elevations.

	PEAK DISCHARGI				<u>GES (CFS)</u>		
FLOODING SOURCE <u>AND LOCATION</u>	DRAINAGE AREA <u>(sq. miles)</u>	10% Annual <u>Chance</u>	2% Annual <u>Chance</u>	1% Annual <u>Chance</u>	0.2% Annual <u>Chance</u>		
ALCORN BAYOU							
Approximately 0.47 mile upstream of confluence with Brazos River	1.20	*	*	579	*		
BRAZOS RIVER							
At the Brazoria / Fort Bend							
County Line	*	103,000	145,000	162,000	200,000		
At US 90A (Richmond Gage)	35,541	103,000	147,000	164,000	202,000		
Just upstream of FM 723	*	103,400	148,500	165,700	201,100		
Just upstream of FM 1093	*	103,900	150,600	168,000	206,900		
At the Waller / Fort Bend							
County line	*	105,400	153,900	171,700	211,500		
BULLHEAD BAYOU							
At US 90A	N/A	*	*	2,191	*		
CANE ISLAND BRANCH							
At River Mile 0.0	26.35	3,529	5,874	7,209	10,451		
At River Mile 0.9	25.90	3,318	4,498	5,172	6,768		
At River Mile 1.2	25.41	3,192	3,794	4,135	4,962		
* Data not computed							

## TABLE 5 – SUMMARY OF DISCHARGES

### TABLE 5 – SUMMARY OF DISCHARGES - Continued

		]	PEAK DISCHARGES (CFS)		
FLOODING SOURCE AND LOCATION	DRAINAGE AREA (sq. miles)	10% Annual Chance	2% Annual Chance	1% Annual Chance	0.2% Annual Chance
CLEAR CREEK	<u></u>				
Approximately 0.42 mile					
downstream of State Route 288	6.17	645	891	1049	1230
At FM 2234	5.27	540	825	969	1160
Approximately 0.62 mile upstream of FM 2234	2.82	438	767	951	1160
upsiteant of 11v1 2234	2.82	430	/0/	931	1100
CLODINE DITCH/LONG POINT SL	OUGH				
At FM 1093	9.62	797	1157	1287	1650
At Harlem Road	7.33	594	855	962	1240
Approximately 1.35 miles					
upstream of Harlem Road	5.25	401	577	683	890
COONCREEK					
COON CREEK At Band Road	14.85	1,250	*	2,400	3,750
Approximately 0.28 mile	14.05	1,230	-	2,400	3,750
upstream of Southern Pacific					
Railroad	11.70	1,100	*	2,400	3,750
Approximately 0.29 mile		· · ·		7	- ,
upstream of Cottonwood Road	10.00	1,000	*	1,700	2,450
DRY CREEK	10.5	1 0 7 7	0 550	2.256	4 100
At Berdett Road	12.5	1,977	2,779	3,356	4,100
At Ricefield Road	9.0	1,693	2,461	2,795	3,650
At FM 2977 Approximately 1,180 feet	8.5	1,644	2,388	2,700	3,500
downstream of Bryan Road	3.88	1,150	1,600	1,800	2,400
Approximately 1,920 feet	5.00	1,150	1,000	1,000	2,400
upstream of Bryan Road	3.53	1,050	1,500	1,650	2,000
		·	·		
NORTH BRANCH DRY CREEK					
At confluence with Dry Creek	0.36	193	*	260	295
Approximately 0.46 mile upstream					
of confluence with Dry Creek	0.31	1.9	*	145	165
KEEGANS BAYOU	8.18	*	*	2 6 1 0	*
At Keegans Road At the Harris- Fort Bend County	8.18 5.1	900	*	3,619 1,700	3,450
Line	J.1	200	•	1,700	5,450
At Belknap Road	3.68	*	*	1,842	*
At Gaines Road	1.88	*	*	600	*
* Data not computed					

### TABLE 5 – SUMMARY OF DISCHARGES - Continued

	PEAK DISCHARGES (CFS			FS)	
FLOODING SOURCE AND LOCATION	DRAINAGE AREA (sq. miles)	10% Annual Chance	2% Annual Chance	1% Annual <u>Chance</u>	0.2% Annual <u>Chance</u>
LITTLE PRONG BUFFALO BAYOU SIDE CHANNEL At confluence with Willow Fork	<u> </u>				
Buffalo Bayou At Katy-Gaston Road	8.65 6.02	1,377 717	* *	2,340 1,256	2,973 1,631
LONG POINT CREEK					
At Briscoe Road At State Route 6	6.56 5.53	784 817	1,062 1,052	1,221 1,162	1,316 1,304
At Dirt Road At Trammel-Fresno Road	3.00 1.60	753 325	955 447	1,047 505	1,160 608
Inflow into Pond B1 Outflow out of Pond B1	1.00 1.03	204 185	258 273	288 310	342 382
Inflow into Pond B3 Outflow out of Pond B3	0.86 0.94	250 176	349 241	397 274	478 320
Inflow into Pond B6 Outflow out of Pond B6	0.36 0.37	78 70	111 97	128 110	154 135
Inflow into Pond B7 Outflow out of Pond B7	0.35 0.36	82 75	118 106	139 122	171 147
LONG POINT CREEK EAST FORK					
Approximately 300 ft upstream confluence with Long Point Creek	1.10	101	202	257	439
At Trammel Fresco	0.83	73	90	96	105
MUSTANG BAYOU At the downstream county					
boundary At the Missouri Pacific Railroad Approximately 0.15 mile	8.96 8.32	779 758	1,072 960	1,174 1,120	1,448 1,380
upstream of Evergreen Road Just downstream of the aqueduct	6.26 1.77	682 269**	920 431**	1,010 511**	1,250 676**
Just downstream of the confluence of the Mustang Bayou Diversion Channel	1.44	133**	236**	282**	386**
Approximately 0.35 mile downstream of Turtle Creek Drive	0.80	327	456	520	654
At the upstream Limit of Detailed Study	0.52	193	272	311	394

\* Data not computed \*\*Decrease due Mustang Bayou Diversion Channel

### TABLE 5 – SUMMARY OF DISCHARGES - Continued

	PEAK DI				DISCHARGES (CFS)		
	DRAINAGE	10%	2%	1%	0.2%		
FLOODING SOURCE <u>AND LOCATION</u>	AREA <u>(sq. miles)</u>	Annual <u>Chance</u>	Annual <u>Chance</u>	Annual <u>Chance</u>	Annual <u>Chance</u>		
MUSTANG BAYOU							
DIVERSION CHANNEL							
Lake Just upstream of Hermann	1.71	450	641	727	921		
Hospital At the confluence with Mustang							
e	1.18	330	449	503	624		
Bayou	1.18	550	449	305	024		
OYSTER CREEK							
At a point approximately 0.63							
miles downstream of Lake							
Olympia Parkway	45.05	3,862	5,743	6,678	8,649		
At a point approximately 0.46							
miles upstream of Lake Olympia	10.50	2 202	<b>5</b> 010	5 0 4 0			
Parkway	43.62	3,382	5,019	5,848	7,598		
At a point approximately 0.14 mile upstream of Hampton Drive	41.65	2,609	3,894	4,551	6,138		
At Cartwright Road	35.20	2,009	2,498	2,925	3,928		
At a point approximately 0.17 mile	55.20	1,000	2,490	2,925	5,920		
downstream of Dulles Avenue	34.08	1,301	1,981	2,319	3,162		
At a point approximately 0.1 mile	0.100	1,001	1,7 01	_,017	0,102		
upstream of Lexington Boulevard	30.17	892	1,291	1,512	1,981		
At US Highway 59	29.71	889	1,297	1,566	2,054		
At a point approximately 0.75							
miles upstream of US							
Highway 90A	28.36	727	1,050	1,216	1,532		
At a point approximately 0.15							
miles upstream of US	27.92	2 0 9 5	2 0 4 9	2 570	1 (94		
Highway 90A	27.82	2,085	3,048	3,579	4,684		
At a point approximately 0.05 miles downstream of							
Harman Road	26.22	1,874	2,523	2,876	3,634		
At a point approximately 1.12	20.22	1,074	2,323	2,870	3,034		
miles upstream of Harman Road	23.04	1,552	1,703	1,770	1,930		
At a point approximately 0.75							
miles downstream of State							
Highway 6	22.34	1,791	2,631	2,955	3,518		
At State Highway 6	19.30	1,382	2,101	2,452	2,865		

IABLE 5 – SUMMARY OF DISCHARGES - Continued         PEAK DISCHARGES (CFS)					
	DRAINAGE	<u>10%</u>	<u>EAK DISC.</u> 2%	<u>HARGES (C.</u> 1%	<u>rs)</u> 0.2%
FLOODING SOURCE	AREA	Annual	2% Annual	Annual	0.2% Annual
AND LOCATION	(sq. miles)	Chance	Chance	<u>Chance</u>	<u>Chance</u>
OYSTER CREEK continued	<u>(sq. miles)</u>	Chance	Chance	Chance	Chance
At a point approximately 1.24					
miles downstream of					
State Highway 6	18.77	1,362	2,032	2,365	3,104
At FM 1464	12.21	1,232	2,032	2,303	3,300
At a point approximately 0.7 miles	12.21	1,232	2,007	2,455	5,500
downstream of State Highway 99	10.27	1,171	1,828	2,226	2,808
At State Highway 99	8.43	958	1,506	1,882	2,374
At a point approximately 1.24					·
miles upstream of Harlem Road	7.00	8.68	1,298	1,637	2,366
At Farmer Street	3.03	492	749	879	1,162
					,
LOWER OYSTER CREEK					
At a point approximately 0.19					
miles downstream of McKeever					
Road	14.66	1,080	1,777	2,143	2,913
At McKeever Road	14.46	1,074	1,767	2,126	2,874
At a point approximately 0.72					
miles upstream of McKeever					
Road	7.24	322	529	633	853
At a point approximately 0.9					
miles upstream of McKeever					
Road	7.11	318	525	629	849
At a point approximately 0.72					
miles downstream of Watts					
Plantation Road	6.97	282	397	449	557
At a point approximately 0.63					
miles downstream of Watts	<b>6 0</b> 0	000	220	261	4.47
Plantation Road	6.30	232	320	361	447
At a point approximately 0.38					
miles downstream of Watts	C 10	017	272	205	246
Plantation Road	6.19 5.79	217	272	295 220	346
At Watts Plantation Road At Trammel Fresno Road	5.78	173 42	208 61	220 69	242 86
At ITAIIIIIEI FIESIIO KOad	5.32	42	01	09	00

### TABLE 5 – SUMMARY OF DISCHARGES - Continued

TABLE 5 – SUMMARY OF DISCHARGES - Continued         PEAK DISCHARGE					FS)
FLOODING SOURCE	DRAINAGE	10%	<u>2%</u>	<u>1%</u>	0.2%
AND LOCATION	AREA	Annual	Annual	Annual	Annual
	<u>(sq. miles)</u>	<b>Chance</b>	<u>Chance</u>	<b>Chance</b>	<b>Chance</b>
RED GULLY At its confluence with Oyster					
Creek	5.70	1,005	1,368	1,561	2,180
Approximately 2.61 miles	0.110	1,000	1,000	1,001	_,100
upstream of its confluence with					
Oyster Creek	2.78	450	575	650	900
Approximately 3.50 miles					
upstream of its confluence with		225	100		
Oyster Creek	2.32	325	490	573	820
SAN BERNARD RIVER					
At FM 442	727	16,100	27,100	32,600	48,000
At downstream confluence with		- ,		- ,	- ,
Snake Creek	726	16,106	27,186	32,100	46,800
At downstream confluence with					
Peach Creek	659	14,855	25,204	29,681	43,400
SEABOURNE CREEK					
Just upstream of J. Meyer Road	6.79	936	1,408	1,632	2,108
Approximately 0.56 mile			·		
downstream of State Route 36	5.69	821	1,227	1,396	1,762
Just downstream of State Route 36	4.39	604	881	1,004	1,313
Approximately 0.29 mile upstream	2.41	550	*	900	1,200
of State Route 59	2.41	550		200	1,200
Approximately 0.13 mile					
upstream of Southern Pacific	1.75	100	*	710	0.40
Railroad At Old US Route 59	1.75 0.99	460 125	*	710 220	840 350
At Scott Road	0.39	54	*	220 95	120
The Scott Road	0.57	54		)5	120
SNAKE CREEK					
At confluence with Willow Fork	12.15	$1,132^{1}$	$1,789^{1}$	$2,148^{1}$	$4,178^{1}$
Bayou	12.15	1,152	1,707	2,140	4,170
SNAKE SLOUGH					
SNAKE SLOUGH Approximately 600 feet					
downstream of Hagerson Road	0.41	*	*	297	*

### TABLE 5 – SUMMARY OF DISCHARGES - Continued

<sup>1</sup> Reduced discharges are due to overflow into Cane Island Watershed and storage routing effects. \* Data not computed

## TABLE 5 – SUMMARY OF DISCHARGES - Continued

TABLE 5 – SUMMARY OF DISCHARGES - Continued					
			PEAK DISC	HARGES (C)	FS)
FLOODING SOURCE	DRAINAGE	10%	2%	1%	0.2%
	AREA	Annual	Annual	Annual	Annual
AND LOCATION	<u>(sq. miles)</u>	<b>Chance</b>	<b>Chance</b>	Chance	Chance
STAFFORD RUN					
At confluence with Oyster Creek	5.18	1,086	1,899	2,218	3,548
Downstream of detention ponds	4.21	954	1,588	1,889	3,162
Upstream of detention ponds	4.00	2,380	3,313	3,816	5,011
At System A Canal	2.90	1,807	2,484	2,787	3,311
At Brand Road	0.32	366	435	550	661
STEEP BANK CREEK					
Approximately 1.1 miles upstream	1.60	*	*	1,491	*
	1.00			1,171	
WILLOW FORK BUFFALO					
BAYOU					
Approximately 1.59 miles					
downstream of the confluence of					
Little Prong Buffalo Bayou	82.89	8,000	*	14,500	22,000
Approximately 0.19 miles	02107	0,000		1 .,000	,
upstream of the confluence of					
Little Prong Buffalo Bayou	66.01	6,600	*	11,700	17,500
Approximately 1.03 miles	00.01	0,000		11,700	17,500
downstream of Greenbush Road	59.95	6,050	*	11,100	16,000
Approximately 0.21 miles		- ,		7	- ,
upstream of Greenbush Road	58.58	6,000	*	11,000	15,850
Approximately 1 mile downstream		-,		,	,
of Crossover Road	54.48	5,700	*	10,400	14,200
Approximately 0.26 mile				- 7	,
downstream of FM 1463	43.38	5,600	*	7,000	10,000
Approximately 0.46 mile		-,		.,	
upstream of FM 1463	20.34	2,319	*	3,654	4,587
Approximately 1.00 mile		y		- ,	<b>y</b>
upstream of FM 1463	18.32	2,121	*	3,326	4,168
Approximately 2.06 mile		,		,	,
upstream of FM 1463	13.35	1,492	*	2,213	2,717
T		· ·		7 -	7
WILLOW FORK DIVERSION					
CHANNEL					
At divergence from Willow Fork					
Buffalo Bayou	*	4,500	6,335	7,810	9,370
Durrato Dajou		1,000	0,000	,,010	2,370
COW CREEK					
Approximately 0.47 miles					
upstream of its confluence with the	61.00	3,600	5,900	6,900	9,100
Brazos River					
* Data not computed					

### TABLE 6: SUMMARY OF STILLWATER ELEVATIONS

### Elevation (ft)

Flooding Source	<u>10-</u> Percent- <u>Annual-</u> Chance	<u>2-Percent-</u> <u>Annual-</u> <u>Chance</u>	<u>1.0-</u> Percent- <u>Annual-</u> <u>Chance</u>	<u>0.2-</u> <u>Percent-</u> <u>Annual-</u> <u>Chance</u>
Alcorn Bayou	*	*	~ * 1	.1.
AB Lake A1	*	*	65.1	* *
AB Lake A2			65.1	
Barker Reservoir	93.8	96.4	97.2	99.0
Brazos River				
West WWTP Interior Drainage	*	*	76.7	*
LID No. 11 Interior Drainage	*	*	68.2	*
Diversion Channel Interior Lakes	*	*	68.2	*
LID No. 17 Interior Drainage				
Main Lake Storage Reservoir	*	*	65.5	*
Tract 5	*	*	68.8	*
LID No. 20 Interior Drainage				
Lake 1	*	*	88.5 <sup>1</sup>	*
Lake 2	*	*	88.5 <sup>1</sup>	*
Lake 3	*	*	88.5 <sup>1</sup>	*
Lake 4	*	*	88.5 <sup>1</sup>	*
Lake 5	*	*	88.5 <sup>1</sup>	*
Longpoint Creek East Fork				
Pond A	67.88	70.03	70.84	72.46
Pond B	67.86	67.00	70.80	72.39
Pond C	67.91	69.71	70.45	71.76
Pond D	67.72	69.75	70.48	71.79
Snake Slough				
SS Lake 1	*	*	64.9	*
Steep Bank Creek				
Riverstone Pond	*	*	64.6	*
SB Lake 81	*	*	64.0	*
SB Lake 82	*	*	63.6	*
SB Lake 83	*	*	63.6	*
SB Pond F	*	*	63.9	*

\* Data not computed <sup>1</sup> Elevation computed using combined probability analysis with Brazos River

#### 3.1.3 Enhanced Approximate Study Streams

In this study, approximately 294 miles of the effective approximate floodplain boundaries were restudied by using the Enhanced Approximate method. ArcHydro Tools were used to derive the contributing drainage areas and appropriate flow change locations (Reference 22). Flows for the 1-percent annual chance flood event were calculated using the USGS regression equation (Reference 23). Fort Bend County is located in Region 11 of Texas and consequently has only one regression equation valid for watersheds of all sizes. The equation is as follows:

Where:

$$Q_{100} = 213*(A^{0.755})*(SL^{0.442})$$

 $Q_{100} = 1$ -percent annual chance discharge (cubic feet per second -cfs) A = Contributing Drainage Area (square mile) SL = Stream Slope (feet per mile)

Enhanced Approximate Study streams are described on Table 7.

#### Flooding Streams Downstream Location Upstream Location **Bessies** Creek Confluence with Brazos River Fort Bend/Waller County Line Confluence of Cottonwood Confluence with Brazos River **Big Creek** and Coon Creeks Confluence with San Bernard River **Brooks Branch** Limit of Approximate Study Harlem Road Pecan Grove M.U.D. Bullhead Bayou Cedar Creek Fort Bend/Brazoria County Line **Bushnell Road** Coon Creek Limit of Detailed Study US Highway 90A Cottonwood Creek Confluence with Big Creek Limit of Approximate Study Dry Creek Smithers Lake Limit of Detailed Study Flewellen Creek Confluence with Jones Creek FM 359 Foss Creek Confluence with Big Creek Limit of Approximate Study Jones Creek Confluence with Brazos River Montgomery Road Rabbs Bayou Limit of Brazos River Detailed Study Limit of Approximate Study San Bernard River Fort Bend/Brazoria County Line Fort Bend/Austin County Line Sandy Branch Confluence with Brazos River Limit of Approximate Study Seabourne Creek Confluence with Big Creek Limit of Detailed Study Snake Creek Confluence with San Bernard River FM 1952 Confluence with San Bernard River US Highway 90A Turkey Creek

### TABLE 7: ENHANCED APPROXIMATE STUDY STREAMS

#### 3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the sources studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the FIRM represent rounded whole-foot elevations and may not exactly reflect the elevations shown on the Flood Profiles or in the Floodway Data tables in the FIS report. Flood elevations shown on the FIRM are primarily intended for flood insurance rating purposes. For construction and/or floodplain management purposes, users are cautioned to use the flood elevation data presented in this FIS in conjunction with the data shown on the FIRM.

Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1). For stream segments for which a floodway was computed (Section 4.2), selected cross-section locations are also shown on the Flood Insurance Rate Map (Exhibit 2).

The hydraulic analyses for this study were based on unobstructed flow. The flood elevations shown on the profiles are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

#### 3.2.1 New Detailed Study Streams

Cross section data for the new detailed analyses of the Brazos River, Oyster Creek, and Lower Oyster Creek were obtained from the combination of field surveyed data within the channels, and LIDAR topography collected in 2005 for the overbanks. USACE HEC-RAS and HEC-GeoRAS computer programs (References 24 and 25) were used as the main hydraulics tools to compute the floodplain elevations and generate the floodplain boundaries. Manning's n values were estimated based on field investigations, field pictures, and aerial photography. The n-values and expansion and contraction coefficients followed recommendations set forth in the USACE HEC- RAS Hydraulic Reference Manual. Ineffective flow areas were carefully analyzed and included as needed at bridges and culverts.

For the Oyster Creek and Lower Oyster Creek studies, eighty four (84) cross-section channels and 53 hydraulics structures were field surveyed. There was no recorded high water mark for calibration of the hydraulics models. The hydraulics models were validated through comparison with the previous FIS study (1977), the Upper and Middle Oyster Creek study by Brown & Gay (BGE) and Costello (2002), and the Missouri City Drainage Master Plan update by Dodson & Associates (2001).

For the Brazos River study, fifty five (55) valley cross-section channels, 10 hydraulics structures, and 5 levee cross sections were field surveyed. Approximately thirty (30) interpolated cross sections were added the hydraulics model to reflect the bridge expansion and contraction; the beginning and the end of the levees; and the significant distance between two surveyed cross sections. The hydraulic model was calibrated against the 1991, 1994, and 1998 flooding events, and validated against the 2007 high flow event. Certified levees along the Brazos River were considered operational in the hydraulic modes.

For Cane Island Branch, Clear Creek (from FM 2234 to the Harris and Fort Bend County line), and Keegan Bayou (from Belknap Road to the Harris and Fort Bend County line); hydraulics analyses were developed as parts of the Harris County 2007 FIS, using the USACE HEC-RAS computer program (Reference 26).

#### 3.2.2 Redelineated Detailed Study Streams

Base Flood Elevations from the 2001 FIS report and LOMR's issued for redelineation streams were used to redelineate the floodplain on the 2005 LiDAR data. In the process of redelineation mapping, there have been overlaps of floodplain information and LOMR's coverage across the county lines of Fort Bend and its adjacent counties (Harris, Waller, and Brazoria). In those overlapping areas, the latest information was utilized to create a seamless floodplain across the county lines. It should be noted that some inconsistencies may still remain; a new derailed study of the entire watershed will be required to clarify all issues.

Fort Bend County has experienced subsidence in the northeast part of the county. The 2001 floodplain was mapped based on the vertical datum of NGVD 1929, 1973 adjustment. For this revision, the vertical datum was converted to NAVD 1988. Datum conversion between NGVD 1929 and NAVD 1988 was considered minor. To avoid a significant discontinuity at the county boundary, subsidence for only those streams that continue into a downstream county were taken into account in the adjustment of the BFE's. The subsidence adjustments are based on published data from Harris Galveston Coastal Subsidence District (HGCSD) and National Geodetic Survey (NGS). Table 8 provides the adjustment values used in the redelineation mapping process.

#### TABLE 8: SUBSIDENCE ADJUSTMENT VALUES FOR REDELINEATED STREAMS

Redelineated Streams	Adjustment (ft)	Adjusted Profiles
Willow Fork Buffalo Bayou	-1.0	65P
Willow Fork Diversion Channel	-1.0	68P
Clodine Ditch/Long Point Slough	-1.0	23P
Red Gully	-1.5	53P
	-1.0	54P
Stafford Run	-2.5	62P
	-2.0	63P
Clear Creek	-2.5	22P
Keegans Bayou	-1.5	34P
Mustang Bayou	-2.0	42P
	-1.0	43P
Mustang Bayou Diversion Channel	-1.0	44P
Long Point Creek	-1.5	38P
Long Point Creek East Fork	-1.5	39P

In the original studies, cross section data for the redelineation streams were obtained from USGS topographic maps; third order leveling was used for Stafford Run, Dry Creek, North Branch Dry Creek, and Seabourne Creek. Cross sections were located at close intervals upstream and downstream of bridges and culverts in order to compute the significant backwater effects of these structures. All bridges and culverts were field surveyed to obtain elevation data and structural geometry.

Water-surface elevations of floods of the selected recurrence intervals were computed using the USACE HEC-2 and/or HEC-RAS step-backwater computer programs. The hydraulic analyses were based on unobstructed flow. The flood elevations shown on the profiles are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail

In the original studies, starting water-surface elevations for the streams studied by detailed methods, except for Willow Fork Buffalo Bayou and North Branch Dry Creek, were calculated by the slope/area method. Starting water-surface elevations for Willow Fork Buffalo Bayou were obtained by critical depth computations. Starting water-surface elevations for North Branch Dry Creek were based on coincident peak with Dry Creek.

Approximate water-surface elevations for Bessies Creek were obtained from normal depth computations. For Long Point Creek, Brookshire Creek, Dry Creek, and Rabbs Bayou, approximate water-surface elevations were obtained from average depth computations. For Jones Creek, approximate water-surface elevations were calculated by the slope/area method.

The hydraulics analysis for Clear Creek was prepared by Dannenbaum Engineering Corporation, dated October 28, 1991, using the USACE HEC-2 computer program. Cross sections for the backwater analysis were obtained from field surveys, highway plans, and aerial photographs. Roughness coefficients (Manning's "n" values) used in the hydraulic computations were chosen by engineering judgment and based on field observations of the stream and floodplain areas.

Analyses of hydraulic characteristics of the San Bernard River were performed by the USACE, Galveston District, dated September 1998. The analyses provide predicted water-surface elevations of floods of the selected exceedance probabilities. Water surface elevations of the 10, 2, 1, 0.2 % exceedance probabilities were computed using the USACE HEC-RAS computer program (Reference 27). Channel sections were obtained by field surveys and the valley sections were taken from the USGS quadrangle maps at a scale of 1:24,000 with a contour interval of 5 feet. Roughness coefficients were estimated for the stream channel and floodplain areas on the basis of field inspections (Reference 28) and orthographic images. The channel roughness coefficient was determined to be 0.07, while overbank roughness coefficients ranged from 0.06 to 0.09. Starting water surface elevations were determined by using normal depth calculations and assuming the slope of the energy grade line equal to the slope of the channel bottom.

Channel and overbank roughness coefficients for the detailed study streams are listed in Table 9.

TABLE 9:	MANNINGS	"'n"	VALUES
----------	----------	------	--------

hannel "n"	Overbank "n"
.030-0.044	0.050-0.120
.040-0.050	0.060-0.200
.013-0.081	0.050-0.015
.015-0.035	0.070
.040-0.070	0.057-0.100
.045-0.055	0.057-0.100
.035-0.045	0.050-0.075
.030-0.050	0.060-0.180
.040-0.150	0.080-0.150
0.50	0.120
0.060	0.120-0.140
0.060	0.120
0.040	0.050-0.090
.060-0.065	0.120-0.130
0.040	0.050-0.100
.035-0.050	0.120
.040-0.050	0.060
.032-0.040	0.060-0.100
.040-0.050	0.120
.015-0.035	0.120
	030-0.044 040-0.050 013-0.081 015-0.035 040-0.070 045-0.055 035-0.045 030-0.050 040-0.150 0.060 0.060 0.060 0.040 060-0.065 0.040 035-0.050 040-0.050 040-0.050

Flood profiles were drawn showing computed water-surface elevations for floods of the selected recurrence intervals. Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1) and are shown on the Digital Flood Insurance Rate Map (DFIRM). The locations of the NGS bench marks are shown on the maps.

#### 3.2.3 Enhanced Approximate Study Streams

The 1-percent annual chance water surface elevations of enhanced approximate study streams were determined by using USACE HEC-RAS computer program (Reference 24). Cross sections for enhanced approximate study streams were obtained from the 2005 LiDAR data. The cross sections were located approximately every 1,500 feet along the stream centerline and all cross sections were oriented perpendicular to the stream flow. The normal depth starting water surface condition was used to initiate the hydraulic computations. No hydraulic structures were included in the study.

Most of studied streams are located in rural areas. Based on aerial imagery of the area

and field reconnaissance information, the overbank roughness coefficients were set between 0.06-0.08. The channels were found to be fairly clean and straight with some vegetation and were assigned a roughness coefficient of 0.035.

Based on the 1-percent annual chance water surface elevations generated by the HEC-RAS models and the LiDAR topographic data, USACE HEC-GeoRAS computer program was used to delineate an initial floodplain boundary. The initial floodplain boundary was then revised and updated to remove areas of inundation that were unrealistic due to lack of hydraulic connectivity and to ensure the overall validity of the floodplain boundaries with respect to local topography.

#### 3.3 Vertical Datum

All FIS reports and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum used for newly created or revised FIS reports and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD). With the completion of the North American Vertical Datum of 1988 (NAVD), many FIS reports and FIRMs are now prepared using NAVD as the referenced vertical datum.

The 2001 floodplain was mapped based on the vertical datum of NGVD, 1973 adjustments. Flood elevations shown in this FIS report and on the DFIRM are referenced to the NAVD (2001 adjustment). These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. The datum conversion factor from NGVD to NAVD in Fort Bend County was -0.014.

For information regarding conversion between the NGVD and NAVD, visit the National Geodetic Survey (NGS) website at <u>www.ngs.noaa.gov</u>, or contact the NGS at the following address:

NGS Information Services, NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282

To obtain current elevation, description, and/or location information for benchmarks shown on this map, please contact the Information Services Branch of the NGS at (301) 713-3242, or visit their website at www.ngs.noaa.gov.

#### 3.4 Land Subsidence

Base flood elevations for most of the flooding sources shown on the DFIRM and in this report were initially developed using benchmarks referenced to the NGVD. Fort Bend County and Incorporated Areas are affected by land subsidence. Land subsidence is the lowering of the ground as a result of water, oil, and gas extraction, as well as other phenomena such as soil compaction, decomposition of organic material, and tectonic movement. Due to the presence of land subsidence, some or all of the benchmarks used to develop the base flood elevations on the FIRM have subsided.

Periodically, the NGS relevels some benchmarks to determine new elevations above the NGVD; however, not all benchmarks are releveled each time. Releveling in Fort Bend County was conducted in 1973, 1979, and 1987. The survey for the 2001 FIS for Fort Bend County was conducted in 1978 using many benchmarks that were established prior to 1973 and may or may not have been releveled in 1973 or later.

The Fort Bend Subsidence District (FB District) was created by the Texas Legislature in 1989 as a conservation and reclamation district (Act of May 26, 1989, 71st Leg., R.S., ch. 1045, 1989 Tex. Gen. Laws 4251). The District's purpose is to provide for the regulation of the withdrawal of groundwater within the District to prevent subsidence that contributes to flooding, inundation or overflow of areas within the District, including rising waters resulting from storms or hurricanes, The District's boundaries are defined as all the territory within Fort Bend County (Reference 29).

Currently, total water use in the FB District is comprised of 60% groundwater and 40% surface water; the surface water, however, is primarily used for manufacturing and agricultural uses. The FB District was formed to address measured impacts from the predominant use of groundwater. In addition to the moderate, but noticeable, amounts of subsidence, the heavy dependence of groundwater has resulted in declining water levels in wells in the aquifers. Groundwater levels in wells drawing from the Chicot and Evangeline Aquifers in the eastern part of the District have declined in excess of of 150 feet from 1943 to 1977. These declines have resulted in increased operational costs to well users.

The Harris-Galveston Coastal Subsidence District (H-GCSD) was created by the Texas Legislature in 1975 as an underground water conservation district for the purpose of controlling subsidence. In 2000, the H-GCSD and NGS conducted a major re-leveling effort in 9-county area (Harris, Fort Bend, Galveston, Brazoria, Waller, Montgomery, Liberty, and Chamber Counties), mostly in Harris County. Updated elevations were established on 181 benchmarks with approximate 25 benchmarks within Fort Bend County. Figure 1 shows the NGS-HGCSD bench marks.

The datum of this network is NAVD 1988 with a vertical height adjustment to 2000. The subsidence map of Harris County from 1973 to 2000 in the Tropical Storm Allison Recovery Project (TSARP) was used to develop the subsidence adjustment values. The recommended values from Table 8 were primarily used in the redelineation mapping process. However, for Stafford Run and Mustang Bayou, the subsidence adjustment values were revised to obtain a more accurate floodplain map. The revised values were based on engineering analysis of the difference between the redelineation map and the 2001 FIRM's. Figure 2 shows the subsidence adjustment values from 1973 to 2000. There is no vertical height adjustment from 2000 to 2001.

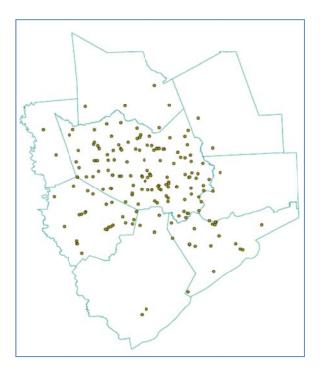


Figure 1: NGS-HGCSD Benchmarks

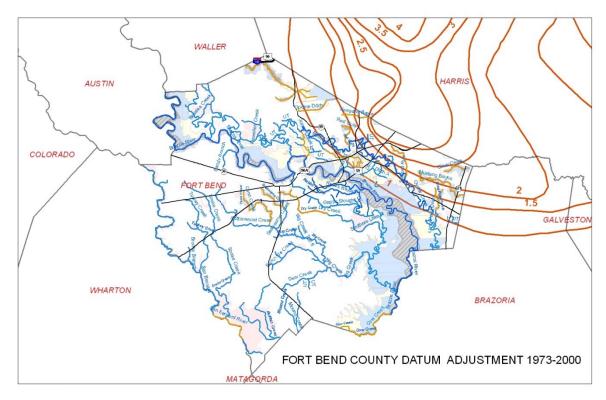


Figure 2: Fort Bend Subsidence Adjustment Values from 1973 to 2000

The prevalence of land subsidence in the study area complicates the determination of the amount a given property lies above or below the base flood elevation. Complicating factors include determining which benchmark releveling to use to determine a property elevation and possible changes in flood hazards as a result of subsidence. Changes in flood hazards, caused by changed hydrologic and hydraulic conditions, could include increases or decreases in (1) depths of flooding, and (2) the amount of land inundated.

A study of subsidence was undertaken by the local entities primarily responsible for water supply and subsidence and flood control in the Houston metropolitan area - Fort Bend County Drainage District, Harris County Flood Control District (HCFCD), Harris-Galveston Coastal Subsidence District (H-GCSD), and the City of Houston. The study, dated December 1986, is entitled "A Study of the Relationship between Subsidence and Flooding" (Reference 30). The effects of subsidence on flooding are discussed below.

Subsidence within inland watersheds has little or no effect on flood depths when the entire watershed, including all hydraulic structures, subsides uniformly. However, differential subsidence (the presence of differing amounts of subsidence within a watershed) can cause changes in stream-channel slope and stream-valley geometry, which can result in changes in flood depths. Where stream-channel slopes are steepened (where there is more subsidence downstream than upstream), flood discharges usually increase and hydraulic efficiency, as measured by the amount of discharge for a given flood depth, increases. In this situation, the depth of flow usually decreases. The opposite is generally true where stream-channel slopes are flattened.

Other effects of land subsidence can include changes in cross-section floodplain geometry and changes in drainage-basin boundaries. Changes in cross-section geometry can affect conveyance, overbank storage, and flow diversions and result in localized changes in flood hazards. Changes in drainage basin boundaries affect the size of the drainage area and can result in changes in discharges and flood depths in the altered basins.

Fort Bend County and Incorporated Areas are affected by relatively wide-scale, uniform subsidence with minor differential subsidence within individual watersheds. Flood depths remain relatively constant and base flood elevations generally subside as the ground subsides (see Figure 3). The local effects of subsidence may be adequately addressed, in the short term, by assuming that base flood elevations subside by the same amount the ground subsides. For floodplain management (setting lowest-floor elevations and regulating construction in the floodplain) and flood insurance (determining the amount the lowest floor of a structure lies above or below the base flood elevation) purposes, the effects of subsidence can be accounted for by determining ground and structure elevations using benchmark elevations with the same relevel date as the benchmarks used to develop the base flood elevations on the FIRM. No adjustment is necessary to the base flood elevations on the FIRM.

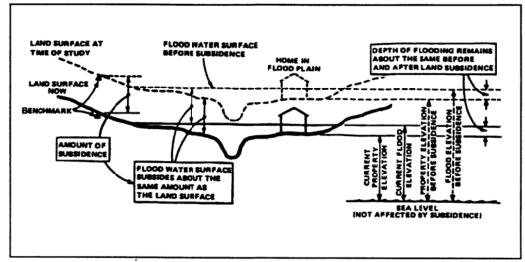


Figure 3: Land Subsidence Schematic

When reviewing development permit applications for new construction in areas subject to ongoing subsidence, consideration should be given to setting the lowest-floor elevation above the base flood elevation by an amount associated with potential increases in flood depths as a result of past and future subsidence. In the absence of site-specific engineering data, elevating a structure by an additional 0.5 foot above the base flood elevation on potential increases in flood depths due to worst-case scenarios of predicted future differential subsidence as discussed in the report entitled "A Study of the Relationship between Subsidence and Flooding." Alternatively, the elevations of more recent releveling of benchmarks, including the 2000 releveling, could be used for ground surveying in setting lowest-floor elevations with the base flood elevations shown on the FIRM.

In watersheds where minor differential subsidence can be considered negligible in the short term, greater differentials in subsidence may occur over time and uniform subsidence assumptions may no longer be appropriate. Additionally, local conditions may produce changes in ground elevations that cannot always be predicted. As a result, more uncertainty is introduced with respect to potential changes in flood depth. The useful life of a FIS is limited and the FIS must eventually be updated. When an entire watershed, or large portions of a watershed, is restudied and the effects of differential subsidence may be significant, it may be appropriate to relevel benchmark elevations at that time, or use the most recently releveled benchmark elevations. The new or more recent benchmark elevations should be used for developing new topography and new cross-section data for hydrologic and hydraulic models.

Information regarding the location and amount of subsidence is available from the H-GCSD in Friendswood, Texas, and the FB Subsidence District in Richmond, Texas. Subsidence information is available for periods of record including 1906-1943, 1943-1964, 1964-1973, 1973-1978, 1978-1987, 1987-1995, and 1995-2000. In areas affected by subsidence, benchmarks that have been installed with the foundation of the benchmark deep in the ground on a nonsubsiding subterranean layer may provide stable benchmark

elevations even though the surrounding ground is subsiding. Several of these types of benchmarks, referred to as "extensometers," are located within Harris County and Incorporated Areas. Information concerning the location and stability of these benchmarks may be obtained from the H-GCSD.

## 4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The NFIP encourages State and local governments to adopt sound floodplain management programs. To assist in this endeavor, each FIS provides 100-year floodplain data, which may include a combination of the following: 10-, 2-, 1-, and 0.2- percent annual chance flood elevations; delineations of the 1- and 0.2-percent annual chance floodplains; and 1% annual chance floodway. This information is presented on the FIRM and in many components of the FIS, including Flood Profiles, Floodway Data tables and Summary of Stillwater Elevation tables. Users should reference the data presented in the FIS as well as additional information that may be available at the local community map repository before making flood elevation and/or floodplain boundary determinations.

#### 4.1 Floodplain Boundaries

To provide a national standard without regional discrimination, the 1-percent annual chance (100-year) flood has been adopted by FEMA as the base flood for floodplain management purposes. The 0.2-percent annual chance (500-year) flood is employed to indicate additional areas of flood risk in the community. For each stream studied by detailed methods, the 1- and 0.2-percent annual chance floodplain boundaries have been delineated using the flood elevations determined at each cross section. Between cross sections of new detailed study streams, the boundaries were interpolated using topographic maps at a scale of 1:12,000, with a contour interval of 2 feet. Between cross sections, the boundaries were interpolated using topographic maps at a scale of 1:nch=1,000 feet, with a contour interval of 2 feet (Reference 31).

The 1-percent and 0.2-percent annual chance floodplain boundaries are shown on the FIRM. On this map, the 1-percent-annual-chance floodplain boundary corresponds to the boundary of the areas of special flood hazards Zones A, AE, AO, and the 0.2-percent-annual-chance floodplain boundary corresponds to the boundary of areas of moderate flood hazards. In cases where the 1- and 0.2-percent-annual-chance floodplain boundary has been shown. Small areas within the floodplain boundaries may lie above the flood elevations but cannot be shown due to limitations of the map scale and/or lack of detailed topographic data.

For the streams studied by enhanced approximate study methods, the boundaries of the 1percent annual chance floodplain were delineated using USACE HEC-GeoRAS computer program. The floodplain boundary widths increased in some areas and decreased in others. The reductions in inundated area were found in floodplain area due to channelization or channel improvements. The increase in floodplain areas were found in the flat regions of the Fort Bend County, especially in the southwest area. This increase in inundated area was minimized in mapping by using depth analysis to eliminate shallow flooding areas with weak hydraulic connections to the main stream channel.

#### 4.2 Floodways

Encroachment on floodplains, such as structures and fill, reduces flood-carrying capacity, increases flood heights and velocities, and increases flood hazards in areas beyond the encroachment itself. One aspect of floodplain management involves balancing the economic gain from floodplain development against the resulting increase in flood hazard. For purposes of the NFIP, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 1-percent-annual-chance floodplain is divided into a floodway and a floodway fringe. The floodway is the channel of a stream, plus any adjacent floodplain areas, that must be kept free of encroachment so that the 1-percent flood can be carried without substantial increases in flood heights. Minimum Federal standards limit such increases to 1.0 foot, provided that hazardous velocities are not produced. The floodways in this study are presented to local agencies as minimum standards that can be adopted directly or that can be used as a basis for additional floodway studies.

The floodways presented in this study were computed on the basis of equal conveyance reduction from each side of the floodplain. Floodway widths were computed at cross sections. Between cross sections, the floodway boundaries were interpolated. The results of these computations are tabulated at selected cross sections (see Table 10, "Floodway Data"). In cases where the floodway and 1-percent annual chance floodplain boundaries are either close together or collinear, only the floodway boundary is shown.

The area between the floodway and 1-percent-annual-chance floodplain boundaries is termed the floodway fringe. The floodway fringe encompasses the portion of the floodplain that could be completely obstructed without increasing the water-surface elevation of the 1-percent flood more than 1.0 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to floodplain development are shown in Figure 4.

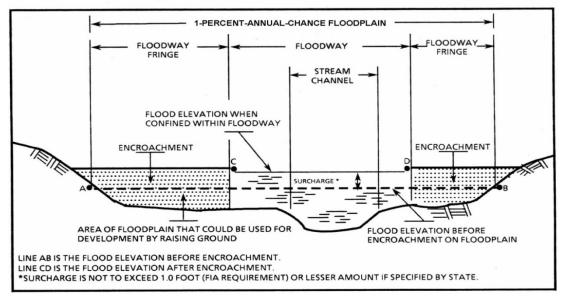


Figure 4: Floodway Schematic

Portions of the floodway widths for the Brazos River, Cow Creek, and Willow Fork Buffalo Bayou extend beyond the county boundary. A floodway was not computed for Willow Fork Diversion Channel.

Near the mouths of streams studied in detail, floodway computations are made without regard to flood elevations on the receiving water body. Therefore, "Without Floodway" elevations presented in Table 10 for certain downstream cross sections of Cow Creek are lower than the regulatory flood elevations in that area, which must take into account the 1-percent-annual-chance flooding due to backwater from other sources.

FLOODING S	DISTANCE <sup>1</sup>	WIDTH (FEET)	FLOODWAY SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	BASE F WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Brazos River								
А	431	8,984/1,922 <sup>2</sup>	89,649	3.4	50.7	50.7	51.7	1.0
В	10,583	6,160/318 <sup>2</sup>	43,995	5.3	51.5	51.5	52.5	1.0
С	10,664	$6,514/298^2$	45,448	5.2	51.6	51.6	52.5	0.9
D	21,940	3,572/814 <sup>2</sup>	26,611	5.3	53.6	53.6	54.3	0.7
Е	29,209	8,986/3,890 <sup>2</sup>	68,310	4.4	54.9	54.9	55.7	0.8
F	41,068	6,654/3,171 <sup>2</sup>	69,681	4.0	56.5	56.5	57.3	0.8
G	48,775	20,620/5,663 <sup>2</sup>	164,444	3.4	57.6	57.6	58.4	0.8
Н	62,793	25,508	201,481	3.2	58.3	58.3	59.3	1.0
I	70,126	12,200	98,871	4.7	58.9	58.9	59.9	1.0
J	78,172	12,834	98,152	4.9	60.4	60.4	61.4	1.0
K	82,824	9,646	85,988	5.1	61.4	61.4	62.2	0.8
L	88,740	751	21,454	8.5	62.9	62.9	63.5	0.6
М	89,013	750	21,790	8.4	63.3	63.3	64.0	0.7
Ν	92,835	7,822	99,258	4.5	64.7	64.7	65.7	1.0
0	106,463	11,390	144,770	3.2	66.1	66.1	66.9	0.8
Р	110,698	13,442	106,480	5.0	66.3	66.3	67.1	0.8
Q	123,453	12,948	96,782	5.2	68.1	68.1	68.7	0.6
R	131,800	10,037	80,754	5.9	69.3	69.3	70.0	0.7
S	136,966	7,472	74,747	6.0	70.2	70.2	71.2	1.0
Т	143,791	8,586	83,432	6.0	71.8	71.8	72.8	1.0
U	147,311	7,502	88,692	4.5	72.6	72.6	73.6	1.0
V	152,185	3,587	59,075	5.6	73.3	73.3	74.2	0.9
W	157,172	3,983	60,243	5.7	74.3	74.3	75.2	0.9

<sup>1</sup> Feet above confluence with Intracoastal Waterway

<sup>2</sup> Total width / width within Fort Bend County

FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

## FLOODWAY DATA

**BRAZOS RIVER** 

TABLE 10

FLOODING S	OURCE	,	FLOODWAY		<b> </b>	BASE F	LOOD	
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
azos River	1					,,		1
(continued)	1		1		1	·   · · · · · · · · · · · · · · · · · ·		1
Х	161,568	3,599	55,681	6.2	75.3	75.3	76.1	0.8
Y	163,253	1,294	31,158	6.6	75.6	75.6	76.4	0.8
Z	163,504	1,295	31,308	6.6	75.8	75.8	76.5	0.7
AA	164,544	3,066	56,875	5.8	76.2	76.2	76.9	0.7
AB	170,554	1,142	28,003	6.4	77.2	77.2	77.9	0.7
AC	170,697	1,143	28,107	6.4	77.3	77.3	78.0	0.7
AD	173,078	1,709	32,738	6.6	78.0	78.0	78.6	0.6
AE	179,826	4,990	91,221	4.0	79.8	79.8	80.3	0.5
AF	184,222	4,136	64,334	5.6	80.1	80.1	80.5	0.4
AG	200,098	2,632	59,282	5.1	81.4	81.4	82.4	1.0
AH	208,418	1,012	22,166	8.8	82.8	82.8	83.7	0.9
AI	208,515	1,012	22,269	8.8	82.9	82.9	83.8	0.9
AJ	208,830	864	22,911	8.2	83.3	83.3	84.1	0.8
AK	209,397	803	20,056	9.3	83.4	83.4	84.2	0.8
AL	212,681	781	26,574	7.2	85.2	85.2	85.8	0.6
AM	223,872	942	29,172	6.6	87.4	87.4	88.3	0.9
AN	232,403	1,181	39,414	4.6	89.0	89.0	89.9	0.9
AO	240,899	2,953	48,716	6.1	91.3	91.3	92.1	0.8
AP	248,700	4,951	54,009	6.1	92.5	92.5	93.3	0.8
AQ	252,398	4,905	57,427	7.0	93.0	93.0	93.9	0.9
AR	252,497	4,982	58,700	6.9	93.2	93.2	94.0	0.8
AS	263,691	4,438	54,193	5.4	94.2	94.2	95.2	1.0
ļ	1		l					I
AS	263,691	4,438	54,193	5.4	94.2	94.2	95.2	

<sup>1</sup> Feet above confluence with Intracoastal Waterway

TABLE 10

FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

## **FLOODWAY DATA**

**BRAZOS RIVER** 

FLOODING S	JOURCE		FLOODWAY SECTION	MEAN	<b> </b>	BASE F		
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	AREA (SQUARE FEET)	VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Brazos River			1				ļ,	
(continued)			1		1		!	1
AT	279,088	6,632	71,363	5.2	95.6	95.6	96.6	1.0
AU	302,479	1,342	34,263	6.7	97.2	97.2	98.2	1.0
AV	322,762	1,000	20,869	9.2	100.7	100.7	101.5	0.8
AW	326,775	1,257	33,767	8.1	102.2	102.2	103.0	0.8
AX	330,774	2,351	43,432	7.0	103.2	103.2	104.1	0.9
AY	342,702	788	22,741	8.8	104.8	104.8	105.7	0.9
AZ	361,344	697	21,192	8.5	108.2	108.2	109.1	0.9
BA	361,439	697	21,248	8.4	108.3	108.3	109.2	0.9
BB	362,916	1,981	30,684	8.2	109.4	109.4	109.8	0.4
BC	366,826	6,634	83,385	5.0	110.1	110.1	111.1	1.0
BD	382,895	9,647/4,658 <sup>2</sup>	126,443	3.6	110.9	110.9	111.9	1.0
BE	397,241	5,881/3,555 <sup>2</sup>	77,758	4.4	111.8	111.8	112.7	0.9
BF	400,911	1,305/815 <sup>2</sup>	29,575	6.9	112.0	112.0	113.0	1.0
BG	401,183	1,305/1,071 <sup>2</sup>	29,689	6.8	112.1	112.1	113.1	1.0
BH	424,653	17,429/7,152 <sup>2</sup>	201,451	2.7	114.1	114.1	115.1	1.0
BI	453,026	16,980/437 <sup>2</sup>	187,999	2.9	114.6	114.6	115.6	1.0
BJ	465,003	8,449/1,857 <sup>2</sup>	58,215	6.8	115.6	115.6	116.6	0.9
BK	468,115	3,509/1,736 <sup>2</sup>	42,245	8.1	116.5	116.5	117.3	0.8

<sup>1</sup> Feet above confluence with Intracoastal Waterway

<sup>2</sup> Total width / width within Fort Bend County

FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS FLOODWAY DATA

**BRAZOS RIVER** 

TABLE 10

FLOODING S	OURCE		FLOODWAY			BASE F	LOOD	
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Cane Island Branch			,	<i>,</i>				
А	543	352 <sup>3</sup>	1,432	3.2	132.0	$128.6^2$	128.9	0.3
В	1,356	350 <sup>3</sup>	1,591	1.9	132.3	$128.9^{2}$	129.3	0.4
С	2,498	348 <sup>3</sup>	1,213	4.5	132.6	129.9 <sup>2</sup>	130.2	0.3
D	2,739	344	1,028	5.5	132.6	130.3 <sup>2</sup>	130.6	0.3
Е	3,219	273	990	5.9	132.6	$131.2^{2}$	131.9	0.7
F	4,258	157	1,248	2.7	133.0	133.0	133.8	0.8
G	4,820	278	1,180	2.9	133.7	133.7	134.2	0.5
Н	5,085	292	1,068	3.2	133.6	133.6	134.6	1.0
Ι	5,961	300	1,581	2.1	135.2	135.2	136.1	0.9
J	6,777	245	1,012	3.3	136.2	136.2	137.0	0.8
К	7,600	260	1,385	2.4	137.8	137.8	138.7	0.9
<sup>1</sup> Feet above confluence with	Willow Fork Buffale	) Bayou		<u> </u>	<u> </u>	<u> </u>	<u> </u>	
<sup>2</sup> Elevation Computed withou		•	ow Fork Buffal	o Bayou				
<sup>3</sup> Floodway width is calculate				-	v Fork Buffalo Bavo	u		
FEDERAL EMERG	FEDERAL EMERGENCY MANAGEMENT AGENCY			FLOODWAY	Y DATA			
FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS				CA	NE ISLAND	BRANCH		

FLOODING S	SOURCE		FLOODWAY			BASE F WATER SURFAC		
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Clear Creek								
А	237,689	850	1124	1.0	59.8	59.8	60.5	0.7
В	238,435	93	440	2.5	60.3	60.3	60.8	0.5
С	239,433	68	362	3.0	60.6	60.6	61.4	0.8
D	239,967	80	528	2.0	60.9	60.9	61.8	0.9
Е	240,152	455	1709	1.0	61.3	61.3	61.7	0.4
F	241,521	484	865	1.3	61.5	61.5	61.8	0.3
G	243,089	472	1319	2.9	62.1	62.1	62.4	0.3
Н	244,941	481	1180	0.9	62.5	62.5	62.8	0.3
Ι	245,978	488	923	1.2	62.7	62.7	63.0	0.3
J	246,832	153	548	2.0	62.9	62.9	63.1	0.2
Κ	247,450	104	704	1.5	63.6	63.6	63.7	0.1
L	249,446	300	455	2.4	64.2	64.2	64.2	0.0
М	250,687	314	1104	0.9	65.3	65.3	65.4	0.1
Ν	253,112	146	756	1.3	67.3	67.3	68.0	0.7
0	255,300	257	470	2.0	67.6	67.6	68.5	0.9
Feet above confluence with	Galveston Bay							
	Garveston Day							
FEDERAL EMERG	ENCY MANAO ND COUNTY		ENCY			FLOODWAY	Y DATA	
	ORPORATE	•		CLEAR CREEK				

FLOODING	SOURCE		FLOODWAY	-		BASE F	LOOD	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Clodine Ditch/Long Point	Slough							
А	5.88 <sup>1</sup>	102	794	2.0	90.6	90.6	90.6	0.0
В	$6.50^{1}$	108	847	1.6	91.0	91.0	91.0	0.0
С	6.59 <sup>1</sup>	108	848	1.6	91.0	91.0	91.0	0.0
D	6.83 <sup>1</sup>	101	843	1.5	91.3	91.3	91.3	0.0
Е	7.21 <sup>1</sup>	97	755	1.5	91.4	91.4	91.4	0.0
F	7.58 <sup>1</sup>	106	895	1.1	91.9	91.9	91.9	0.0
G	7.96 <sup>1</sup>	103	793	1.2	92.0	92.0	92.0	0.0
Н	8.53 <sup>1</sup>	383	785	1.2	92.3	92.3	92.3	0.0
Ι	9.10 <sup>1</sup>	96	714	0.7	92.4	92.4	92.4	0.0
J	9.75 <sup>1</sup>	88	389	1.0	92.5	92.5	92.5	0.0
K	10.18 <sup>1</sup>	52	204	1.6	92.9	92.9	92.9	0.0
L	10.52 <sup>1</sup>	49	202	1.1	94.1	94.1	94.1	0.0
Coon Creek								
А	0.64 <sup>2</sup>	72	364	6.6	81.9	81.9	82.8	0.9
В	0.66 <sup>2</sup>	58	364	6.6	82.7	82.7	83.2	0.5
С	$0.78^{2}$	92	426	5.5	85.5	85.5	85.6	0.1
D	1.19 <sup>2</sup>	200	786	3.0	89.3	89.3	90.0	0.7
Е	1.51 <sup>2</sup>	252	1,123	2.1	90.9	90.9	91.7	0.8
F	1.95 <sup>2</sup>	300	1,415	1.7	92.1	92.1	93.0	0.9
G	$2.20^{2}$	300	1,195	2.0	92.5	92.5	93.4	0.9
Н	2.45 <sup>2</sup>	115	1,049	2.2	93.2	93.2	94.0	0.8
Ι	2.74 <sup>2</sup>	300	1,420	1.4	93.8	93.8	94.5	0.7
J	3.09 <sup>2</sup>	70	610	3.3	94.7	94.7	95.3	0.6
K	3.37 <sup>2</sup>	400	1,225	1.4	95.5	95.5	96.2	0.7
L	$3.77^{2}$	285	837	1.8	96.2	96.2	97.2	1.0

<sup>1</sup> Miles above confluence with Buffalo Bayou

TABLE

10

<sup>2</sup> Miles above confluence with Cottonwood Creek

# FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

## FLOODWAY DATA

**CLODINE DITCH/LONG POINT SLOUGH - COON CREEK** 

FLOODING S	OURCE		FLOODWAY	1		BASE F	LOOD	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Coon Creek			,					
(continued)								
Μ	4.17 <sup>1</sup>	325	1,086	1.4	97.7	97.7	98.3	0.6
Ν	4.53 <sup>1</sup>	355	782	1.9	98.8	98.8	99.4	0.6
0	$4.84^{1}$	355	789	1.9	99.3	99.3	100.3	1.0
Р	5.18 <sup>1</sup>	125	498	3.0	100.6	100.6	101.3	0.7
Q	5.73 <sup>1</sup>	66	469	3.2	102.6	102.6	103.1	0.5
R	5.96 <sup>1</sup>	60	501	3.0	103.1	103.1	103.7	0.6
S	$6.54^{1}$	132	698	2.0	105.3	105.3	106.1	0.8
Т	$6.76^{1}$	150	761	1.5	106.8	106.8	107.4	0.6
U	$6.85^{1}$	400	1,509	0.8	106.9	106.9	107.6	0.7
V	$7.09^{1}$	500	1,595	0.7	107.0	107.0	107.8	0.8
W	7.45 <sup>1</sup>	700	1,388	0.8	107.3	107.3	108.2	0.9
Х	7.72 <sup>1</sup>	700	2,036	0.6	107.6	107.6	108.4	0.8
Cow Creek								
А	$2,480^2$	217 <sup>3</sup>	2,084	3.3	51.0	$29.6^4$	30.6	1.0
В	$5,410^2$	137 <sup>3</sup>	1,340	4.6	51.0	$32.0^{4}$	32.5	0.5
С	9,260 <sup>2</sup>	94 <sup>3</sup>	987	6.3	51.0	$35.2^4$	35.4	0.2
D	$12,620^2$	$120^{3}$	1,256	4.5	51.0	38.1 <sup>4</sup>	38.2	0.1
E	$15,320^2$	96 <sup>3</sup>	993	5.6	51.0	$41.0^{4}$	41.2	0.2
F	$19,070^2$	353 <sup>3</sup>	2,072	2.3	51.0	44.4 <sup>4</sup>	45.0	0.6
G	23,240 <sup>2</sup>	287 <sup>3</sup>	1,746	2.5	51.0	46.4 <sup>4</sup>	47.3	0.9
Ailes above confluence wit	- Cottonwood Croal	_						
Feet above confluence with		Υ.						
This width extends beyond								
Elevation computed without		ckwater effects from	m Brazos River					
	DERAL EMERGENCY MANAGEMENT AGENCY					FLOODWAY	Y DATA	
FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS			COON CREEK - COW CREEK					

FLOODING S	SOURCE		FLOODWAY	-	BASE FLOOD				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)	
Cow Creek			,	, , , , , , , , , , , , , , , , , , ,					
(continued)									
Н	30,140 <sup>1</sup>	309	1,637	2.3	51.0	50.3 <sup>3</sup>	51.2	0.9	
Ι	31,140 <sup>1</sup>	1,052	4,477	0.8	51.0	50.7 <sup>3</sup>	51.6	0.9	
J	33,190 <sup>1</sup>	450	2,263	1.7	51.1	51.1	52.0	0.9	
К	35,260 <sup>1</sup>	547	2,489	1.5	51.9	51.9	52.8	0.9	
L	$37,700^{1}$	527	2,574	1.5	52.8	52.8	53.7	0.9	
М	39,400 <sup>1</sup>	628	2,680	1.4	53.4	53.4	54.4	1.0	
Ν	$40,680^{1}$	456	2,353	1.6	53.9	53.9	54.9	1.0	
0	42,900 <sup>1</sup>	907	4,468	0.9	54.4	54.4	55.4	1.0	
Dry Creek									
А	$0^2$	138	1,420	2.9	74.1	74.1	75.1	1.0	
В	$3,000^2$	153	1,475	2.8	75.0	75.0	75.8	0.8	
С	$5,800^2$	128	1,116	3.4	75.9	75.9	76.5	0.6	
D	$8,800^2$	116	953	3.9	77.4	77.4	77.8	0.4	
Е	$12,000^2$	112	894	4.2	79.5	79.5	79.7	0.2	
F	$13,800^2$	110	916	3.7	80.6	80.6	80.7	0.1	
G	15,340 <sup>2</sup>	109	882	3.9	81.4	81.4	81.5	0.1	
Н	$16,800^2$	76	636	5.2	82.4	82.4	82.5	0.1	
Ι	16,940 <sup>2</sup>	122	1,183	2.8	82.8	82.8	82.9	0.1	
J	19,840 <sup>2</sup>	125	993	3.0	83.8	83.8	83.8	0.0	
K	$22,840^2$	118	885	2.8	84.8	84.8	84.8	0.0	
L	$24,640^2$	122	949	2.6	85.3	85.3	85.3	0.0	
К	26,240 <sup>2</sup>	127	998	2.3	85.7	85.7	85.7	0.0	
Ν	$27,440^2$	130	927	1.9	85.9	85.9	85.9	0.0	
0	28,563 <sup>2</sup>	95	636	2.8	86.1	86.1	86.1	0.0	

<sup>1</sup> Feet above confluence with Brazos River

TABLE 10

<sup>2</sup> Feet above downstream face of Berdett Road

<sup>3</sup> Elevation computed without consideration of backwater effects from Brazos River

FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS FLOODWAY DATA

**COW CREEK - DRY CREEK** 

FLOODING	SOURCE		FLOODWAY			BASE F	LOOD	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Dry Creek								
(continued)								
Р	28,725 <sup>1</sup>	106	615	2.9	86.3	86.3	86.3	0.0
Q	29,885 <sup>1</sup>	91	423	4.3	87.0	87.0	87.0	0.0
R	30,535 <sup>1</sup>	85	390	4.2	87.9	87.9	87.9	0.0
S	30,997 <sup>1</sup>	79	375	4.4	88.5	88.5	88.5	0.0
Т	32,147 <sup>1</sup>	76	425	3.5	89.4	89.4	89.4	0.0
U	32,217 <sup>1</sup>	76	426	3.5	89.4	89.4	89.4	0.0
V	33,867 <sup>1</sup>	53	220	6.8	91.4	91.4	91.4	0.0
W	35,694 <sup>1</sup>	72	347	4.3	94.4	94.4	94.4	0.0
Х	36,106 <sup>1</sup>	72	502	2.5	95.7	95.7	95.9	0.2
Y	37,456 <sup>1</sup>	14	568	2.2	95.7	95.7	96.6	0.9
Z	37,484 <sup>1</sup>	74	578	2.2	96.3	96.3	96.6	0.3
AA	39,484 <sup>1</sup>	75	450	2.0	96.7	96.7	97.5	0.8
AB	39,514 <sup>1</sup>	75	459	2.0	96.8	96.8	97.6	0.8
AC	41,664 <sup>1</sup>	277	739	0.8	97.6	97.6	98.3	0.7
AD	43,314 <sup>1</sup>	145	426	1.5	97.9	97.9	98.8	0.9
AE	44,952 <sup>1</sup>	112	304	2.0	99.1	99.1	100.0	0.9
AF	46,002 <sup>1</sup>	137	563	0.6	99.3	99.3	100.2	0.9
North Branch Dry Creek								
А	1.21 <sup>2</sup>	31	139	1.9	96.3	96.3	97.3	1.0
В	2.46 <sup>2</sup>	32	124	1.2	97.2	97.2	97.8	0.6
С	$2.50^{2}$	25	144	1.0	98.5	98.5	99.0	0.5
D	$3.60^{2}$	31	168	0.9	98.6	98.6	99.1	0.5

<sup>1</sup> Feet above downstream face of Berdett Road

<sup>2</sup> Thousands of feet above confluence with Dry Creek

# FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

## FLOODWAY DATA

DRY CREEK - NORTH BRANCH DRY CREEK

TABLE 10

FLOODING	SOURCE		FLOODWAY			BASE F	LOOD	
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Keegans Bayou								
А	35,267	113	999	1.4	84.4	84.4	85.4	1.0
В	35,780	118	1,029	1.3	84.5	84.5	85.5	1.0
С	36,780	114	1,044	1.3	84.6	84.6	85.6	1.0
D	37,780	115	1,017	1.4	84.7	84.7	85.6	0.9
E	38,780	102	912	1.5	84.8	84.8	85.7	0.9
F	38,887	100	929	1.5	84.8	84.8	85.8	1.0
G	39,780	140	1,067	1.3	84.9	84.9	85.9	1.0
Н	40,780	115	1,007	1.4	85.1	85.1	86.1	1.0
Ι	41,894	105	953	1.0	85.2	85.2	86.1	0.9
J	42,074	105	818	1.1	85.2	85.2	86.1	0.9
K	42,780	110	859	1.1	85.2	85.2	86.1	0.9
L	44,330	100	752	1.2	85.3	85.3	86.2	0.9
K	45,830	96	731	1.3	85.5	85.5	86.4	0.9
Ν	46,830	100	709	1.3	85.6	85.6	86.4	0.8
0	48,530	87	595	1.0	85.8	85.8	85.8	0.0
Р	49,537	85	570	1.1	85.8	85.8	85.8	0.0
Q	50,830	83	549	0.8	85.8	85.8	85.8	0.0
R	51,980	81	518	0.8	85.8	85.8	85.8	0.0
Feet above confluence with	Brays Bayou			1			<u> </u>	
FEDERAL EMERGENCY MANAGEMENT AGENCY					FLOODWAY	Y DATA		
	FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS				]	<b>KEEGANS</b>	BAYOU	

CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Little Prong			FEE1)	SECOND)				
Buffalo Bayou								
Side Channel								
А	3,400 <sup>1</sup>	104	848	2.8	105.8	105.8	105.8	0.0
В	4,480 <sup>1</sup>	160	1,159	1.8	106.2	106.2	106.2	0.0
С	$5,480^{1}$	109	848	2.5	106.5	106.5	106.5	0.0
D	$5,580^{1}$	135	870	2.5	106.5	106.5	106.5	0.0
E	$6,180^{1}$	220	1,756	1.2	106.7	106.7	106.7	0.0
F	7,380 <sup>1</sup>	151	993	2.2	106.9	106.9	106.9	0.0
G	9,140 <sup>1</sup>	107	621	3.5	107.6	107.6	107.6	0.0
Н	$10,240^{1}$	96	594	3.6	108.6	108.6	108.6	0.0
Ι	11,640 <sup>1</sup>	118	622	3.4	110.2	110.2	110.2	0.0
Long Point Creek								
A	$3,010^2$	295	2,011	0.6	58.8	58.8	59.7	0.9
В	$5,510^2$	118	746	2.8	59.6	59.6	60.4	0.8
С	$7,039^2$	125	862	1.4	62.0	62.0	62.7	0.7
D	8,679 <sup>2</sup>	80	541	1.9	62.6	62.6	63.2	0.6
Е	9,869 <sup>2</sup>	48	367	2.4	63.4	63.4	64.2	0.8
F	$11,429^2$	84	605	0.8	64.3	64.3	65.3	1.0
G	$12,239^2$	98	596	0.9	64.4	64.4	65.3	0.9
Н	$12,996^2$	25	245	2.1	65.5	65.5	66.3	0.8
Ι	$15,350^2$	340	3,012	0.1	67.4	67.4	67.4	0.0
J	$17,000^2$	80	725	0.2	67.6	67.6	67.6	0.0
Κ	17,785 <sup>2</sup>	500	4,067	0.03	67.6	67.6	67.6	0.0

<sup>2</sup> Feet above Briscoe Canal

TABLE 10

# FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

# FLOODWAY DATA

LITTLE PRONG BUFFALO BAYOU SIDE CHANNEL - LONG POINT CREEK

FLOODING S	OURCE		FLOODWAY			BASE F	LOOD	
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Long Point Creek			,					
East Fork								
А	330	122	274	0.9	63.5	63.1 <sup>2</sup>	63.1	0.0
В	893	53	234	2.1	63.5	63.3 <sup>2</sup>	63.3	0.0
С	1,193	54	242	1.4	63.7	63.7	63.7	0.0
D	1,945	51	214	1.6	63.9	63.9	63.9	0.0
Е	2,403	53	228	0.6	64.7	64.7	64.7	0.0
F	2,993	49	203	0.5	64.8	64.8	64.8	0.0
<sup>1</sup> Feet above confluence with <sup>2</sup> Elevation computed withou		ckwater effects from	n Long Point C	reek				
	FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS					FLOODWAY	Y DATA	
				LONG POINT CREEK EAST FORK			₹K	

FLOODING S	DISTANCE <sup>1</sup>	WIDTH (FEET)	FLOODWAY SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	BASE F WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Mustang Bayou								
А	0.17	400	1,110	1.1	68.0	68.0	69.0	1.0
В	0.57	400	1,251	0.9	68.6	68.6	69.6	1.0
С	0.93	60	377	3.1	69.1	69.1	69.9	0.8
D	0.96	54	418	2.8	69.4	69.4	69.8	0.4
E	0.97	54	428	2.7	69.6	69.6	70.0	0.4
F	1.40	1,248	3,005	0.4	69.9	69.9	70.6	0.7
G	1.61	975	4,060	0.3	69.9	69.9	70.6	0.7
Н	1.85	935	3,346	0.3	69.9	69.9	70.7	0.8
Ι	2.24	1,070	4,019	0.3	70.0	70.0	70.8	0.8
J	2.37	788	3,217	0.3	70.0	70.0	70.9	0.9
Κ	2.56	500	1,879	0.6	70.1	70.1	71.0	0.9
L	2.71	350	1,642	0.1	70.2	70.2	71.2	1.0
М	3.05	502	1,850	0.5	70.4	70.4	71.4	1.0
Ν	3.41	245	1,469	0.7	70.7	70.7	71.7	1.0
0	3.60	540	1,999	0.5	70.9	70.9	71.9	1.0
Р	4.21	630	1,396	0.9	70.9	70.9	71.0	0.1
Q	4.57	560	1,421	0.8	71.1	71.1	71.2	0.1
R	4.74	700	1,388	0.9	71.1	71.1	71.3	0.2
S	5.19	1,270	1,836	0.7	71.3	71.3	71.7	0.4
Т	5.23	1,194	1,913	0.7	71.4	71.4	71.7	0.3
U	5.50	1,270	3,394	0.3	71.4	71.4	71.8	0.4
V	5.58	889	1,232	1.9	71.5	71.5	71.8	0.3
W	6.02	1,110	3,301	0.3	71.5	71.5	71.9	0.4

<sup>1</sup> Miles above county boundary

TABLE 10

FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

# FLOODWAY DATA

**MUSTANG BAYOU** 

FLOODING S	OURCE		FLOODWAY			BASE F	LOOD	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
/ustang Bayou								
(continued)								
Х	$6.06^{1}$	840	1,018	0.6	71.5	71.5	71.9	0.4
Y	6.19 <sup>1</sup>	417	804	0.3	71.6	71.6	71.9	0.3
Z	6.44 <sup>1</sup>	155	350	0.9	71.6	71.6	72.0	0.4
AA	6.71 <sup>1</sup>	61	267	1.1	72.7	72.7	73.1	0.4
AB	6.96 <sup>1</sup>	71	308	0.9	72.8	72.8	73.2	0.4
AC	7.91 <sup>1</sup>	221	684	1.0	73.3	73.3	73.6	0.3
AD	$8.40^{1}$	262	623	0.8	73.5	73.5	73.9	0.4
AE	8.63 <sup>1</sup>	66	372	0.8	73.5	73.5	74.0	0.5
ower Oyster Creek								
А	3,363 <sup>2</sup>	136	840	0.8	59.2	59.2	59.2	0.0
В	7,169 <sup>2</sup>	182	862	0.5	59.5	59.5	59.5	0.0
С	10,081 <sup>2</sup>	111	554	0.5	59.6	59.6	59.6	0.0
D	11,777 <sup>2</sup>	136	861	0.3	59.6	59.6	59.6	0.0
Е	14,673 <sup>2</sup>	186	1,010	0.1	60.3	60.3	60.3	0.0
F	16,792 <sup>2</sup>	190	691	0.1	60.3	60.3	60.3	0.0
G	18,787 <sup>2</sup>	164	893	0.1	60.7	60.7	60.7	0.0
Н	24,042 <sup>2</sup>	212	1,011	0.1	60.7	60.7	60.7	0.0
Ι	26,653 <sup>2</sup>	144	678	0.1	60.7	60.7	60.7	0.0
Ι	26,653 <sup>2</sup>	144	678	0.1	60.7	60.7	60.7	

<sup>1</sup> Miles above county boundary

<sup>2</sup> Feet above Long Point Creek junction

FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

## FLOODWAY DATA

MUSTANG BAYOU - LOWER OYSTER CREEK

TABLE 10

FLOODING S	SOURCE		FLOODWAY			BASE F	LOOD	
CROSS SECTION	<b>DISTANCE</b> <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Oyster Creek								
А	32,829	160	2,234	2.6	61.5	61.5	61.6	0.1
В	35,238	180	2,113	2.2	61.8	61.8	61.9	0.1
С	37,630	143	2,062	2.2	62.1	62.1	62.2	0.1
D	39,263	172	2,196	2.1	62.3	62.3	62.4	0.1
E	42,102	191	1,981	1.5	62.7	62.7	62.8	0.1
F	45,336	227	2,277	1.3	62.9	62.9	63.0	0.1
G	51,566	215	1,629	1.8	63.6	63.6	63.7	0.1
Н	54,417	195	1,590	1.5	64.0	64.0	64.0	0.0
Ι	57,103	151	1,347	1.1	64.2	64.2	64.3	0.1
J	59,570	217	816	1.9	64.7	64.7	64.7	0.0
Κ	62,772	135	827	1.8	65.5	65.5	65.6	0.1
L	66,651	111	722	2.1	66.3	66.3	66.5	0.2
Μ	69,174	112	759	2.0	66.8	66.8	67.0	0.2
Ν	72,960	251	1,394	1.1	70.4	70.4	70.4	0.0
0	75,046	241	1,548	1.0	70.6	70.6	70.6	0.0
Р	76,558	241	1,570	1.0	70.6	70.6	70.7	0.1
Q	79,477	246	1,577	1.0	70.8	70.8	70.8	0.0
R	80,450	232	1,492	1.1	70.9	70.9	71.0	0.1
S	83,603	284	1,887	0.6	71.2	71.2	71.2	0.0
Т	86,535	190	1,518	0.8	71.2	71.2	71.3	0.0
U	89,046	255	2,509	0.5	71.3	71.3	71.3	0.0
V	91,876	371	1,866	1.9	72.6	72.6	72.6	0.0
W	93,773	273	2,030	1.8	72.9	72.9	72.9	0.0
Х	97,425	273	2,417	1.2	74.6	74.6	74.6	0.0
Y	98,595	263	2,006	0.9	74.8	74.8	74.8	0.0
Z	105,171	241	1,506	2.4	77.2	77.2	77.2	0.0

<sup>1</sup> Feet above Long Point Creek junction

TABLE 10

FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

## FLOODWAY DATA

**OYSTER CREEK** 

FLOODING	SOURCE		FLOODWAY			BASE F	LOOD	
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Oyster Creek								
(continued)								
AA	109,489	307	1,357	2.4	78.4	78.4	78.6	0.2
AB	113,389	350	1,683	1.6	78.9	78.9	79.4	0.5
AC	115,935	505	2,183	1.5	79.0	79.0	79.8	0.8
AD	121,767	284	1,752	1.5	80.0	80.0	80.7	0.7
AE	127,636	428	2,479	1.0	80.5	80.5	81.3	0.8
AF	130,430	279	2,155	1.2	80.6	80.6	81.4	0.8
AG	133,190	264	2,198	1.3	80.8	80.8	81.5	0.7
AH	136,242	214	1,920	1.5	81.0	81.0	81.7	0.7
AI	140,703	237	2,175	1.0	81.4	81.4	82.2	0.8
AJ	145,391	177	1,851	1.4	81.6	81.6	82.4	0.8
AK	148,924	134	1,298	1.4	81.8	81.8	82.6	0.8
AL	152,313	171	1,578	1.1	82.1	82.1	82.8	0.7
AM	155,989	133	1,359	1.3	82.3	82.3	83.1	0.8
AN	160,827	136	1,209	1.5	82.7	82.7	83.4	0.7
AO	165,219	139	1,008	0.8	82.9	82.9	83.6	0.7
AP	169,040	121	1,081	0.8	83.0	83.0	83.8	0.8
AQ	173,067	229	1,658	0.3	83.1	83.1	83.9	0.8
AR	175,317	229	1,660	0.3	83.2	83.2	83.9	0.7
AS	181,667	183	1,206	0.4	85.3	85.3	86.0	0.7
Feet above Long Point Cre	ek junction							
FEDERAL EMERO			NCY			FLOODWAY	Y DATA	
	ND COUNTY	,		OYSTER CREEK				

FLOODING S	OURCE		FLOODWAY	-		BASE F	LOOD	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Red Gully								
А	581 <sup>1</sup>	114	898	1.7	80.2	76.6 <sup>3</sup>	77.6	1.0
В	$620^{1}$	111	882	1.8	80.2	76.7 <sup>3</sup>	77.7	1.0
С	$4,200^{1}$	98	813	1.5	80.2	$77.0^{3}$	78.0	1.0
D	$5,300^{1}$	120	967	1.3	80.2	77.2 <sup>3</sup>	78.2	1.0
Е	$7,000^{1}$	142	1,094	1.0	80.2	77.4 <sup>3</sup>	78.3	0.9
F	7,918 <sup>1</sup>	96	684	2.5	80.2	77.7 <sup>3</sup>	78.4	0.7
G	9,918 <sup>1</sup>	103	785	1.4	80.2	78.4 <sup>3</sup>	78.9	0.5
Н	11,533 <sup>1</sup>	140	928	1.2	80.2	78.4 <sup>3</sup>	79.0	0.6
Ι	13,717 <sup>1</sup>	185	633	1.7	81.2	81.2	81.5	0.3
J	$16,008^{1}$	60	251	2.6	81.9	81.9	82.2	0.3
K	18,490 <sup>1</sup>	412	825	2.0	82.4	82.4	82.9	0.5
L	19,300 <sup>1</sup>	55	192	2.9	82.5	82.5	83.1	0.6
San Bernard River								
А	$10,800^2$	2,812/6124	13,826	3.6	63.7	63.7	64.7	1.0
В	$19,300^2$	574/374 <sup>4</sup>	10,716	3.0	68.1	68.1	69.1	1.0
С	$22,000^2$	370/150 <sup>4</sup>	7,617	4.3	70.0	70.0	70.8	0.8
D	$30,200^2$	$471/200^4$	7,983	4.0	75.7	75.7	76.2	0.5
Е	$38,400^2$	4,221/3,521 <sup>4</sup>	24,925	2.6	78.1	78.1	79.1	1.0
F	43,900 <sup>2</sup>	6,429/6,100 <sup>4</sup>	44,349	1.2	79.1	79.1	80.1	1.0
Feet above confluence with	Oyster Creek		4	+ Total width/wid	th within Fort Bend	County		
Feet above Atchison, Topek Elevation computed without		5	n Oyster Creek	-				
FEDERAL EMERG	ENCY MANAG ND COUNTY		ENCY			FLOODWAY	Y DATA	
	ORPORATE				RED GUL	LY - SAN B	ERNARD R	IVER

FLOODING S	OURCE		FLOODWAY			BASE F	LOOD	
CROSS SECTION	<b>DISTANCE</b> <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Seabourne Creek								
А	3.95	110	708	1.3	93.1	93.1	93.9	0.8
В	4.10	110	689	1.4	93.2	93.2	93.9	0.7
С	4.29	109	608	1.5	93.3	93.3	94.1	0.8
D	4.46	80	609	1.4	93.5	93.5	94.2	0.7
E	4.57	50	586	1.4	93.6	93.6	94.2	0.6
F	4.84	70	198	3.9	94.0	94.0	94.5	0.5
G	5.16	37	262	2.7	96.8	96.8	97.6	0.8
Н	5.38	110	212	3.3	98.2	98.2	99.0	0.8
Ι	5.41	50	216	3.2	98.4	98.4	99.2	0.8
J	5.58	134	363	2.0	99.4	99.4	100.2	0.8
K	5.85	339	561	1.3	100.4	100.4	101.4	1.0
L	6.19	287	850	0.8	100.9	100.9	101.9	1.0
К	6.56	49	185	1.2	102.2	102.2	102.7	0.5
Ν	6.60	46	179	1.2	102.2	102.2	103.2	1.0
0	6.80	114	195	1.1	102.7	102.7	103.5	0.8
Р	7.14	177	307	0.7	103.1	103.1	104.1	1.0
Q	7.44	17	47	2.0	103.6	103.6	104.6	1.0
R	7.49	103	210	0.5	104.1	104.1	104.9	0.8
Miles above confluence wit	n Big Creek							
FORT BE	FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS				a	FLOODWAY		
AND INC	UNFUKATE	U AREAS		SEABOURNE CREEK				

Γ	FLOODING SOURCE H		FLOODWAY			BASE F	LOOD		
	CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
	Tributary 20.25 to Sims Bayou A B	8,040 8,407	83 80	587 373	0.1 0.2	59.2 59.2	59.2 59.2	60.1 60.1	0.9 0.9
1 ]	Feet above confluence with	Sims Bayou			<b></b>				
		ENCY MANAG ND COUNTY ORPORATE	Y, TEXAS	ENCY			FLOODWAY	Y DATA O SIMS BA	YOU

FLOODING S	SOURCE		FLOODWAY			BASE FI	LOOD	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Snake Creek								
А	1.356 <sup>1</sup>	391	975	2.2	136.5	133.4 <sup>3</sup>	133.9	0.5
Stafford Run								
А	1,069 <sup>2</sup>	63	295	7.5	62.4	$60.2^4$	60.2	0.0
В	3,019 <sup>2</sup>	62	357	5.8	62.4	61.1 <sup>4</sup>	61.1	0.0
С	4,624 <sup>2</sup>	74	330	6.1	62.4	61.8 <sup>4</sup>	61.8	0.0
D	4,724 <sup>2</sup>	71	300	6.7	62.4	61.8 <sup>4</sup>	61.8	0.0
Е	5,624 <sup>2</sup>	92	438	4.5	62.8	62.8	62.8	0.0
F	5,789 <sup>2</sup>	82	485	4.0	63.0	63.0	63.0	0.0
G	$7,060^2$	67	372	5.1	63.0	63.0	63.0	0.0
Н	8,497 <sup>2</sup>	210	1,958	1.8	67.5	67.5	67.5	0.0
Ι	$10,262^2$	174	1,722	1.9	68.1	68.1	68.1	0.0
J	12,137 <sup>2</sup>	75	609	5.0	68.5	68.5	68.5	0.0
К	12,197 <sup>2</sup>	75	612	5.0	68.6	68.6	68.6	0.0
L	13,121 <sup>2</sup>	104	642	3.5	69.7	69.7	69.7	0.0
М	14,175 <sup>2</sup>	93	767	3.6	70.3	70.3	70.3	0.0
Ν	14,215 <sup>2</sup>	95	820	3.4	70.6	70.6	70.6	0.0
0	14,230 <sup>2</sup>	95	820	3.4	70.6	70.6	70.6	0.0

<sup>2</sup> Feet above confluence with Oyster Creek

TABLE 10

<sup>4</sup> Elevation computed without consideration of backwater effects from Oyster Creek

<sup>3</sup> Elevation computed without consideration of backwater effects from Willow Fork Buffalo Bayou

# FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

# FLOODWAY DATA

# **SNAKE CREEK - STAFFORD RUN**

FLOODING S	SOURCE		FLOODWAY			BASE F	LOOD	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Stafford Run			,	, , , , , , , , , , , , , , , , , , ,				
(continued)								
Р	$14,580^{1}$	71	552	4.8	70.6	70.6	70.6	0.0
Q	15,906 <sup>1</sup>	69	522	4.4	70.8	70.8	70.8	0.0
R	17,318 <sup>1</sup>	100	667	2.6	71.3	71.3	71.3	0.0
S	18,612 <sup>1</sup>	115	757	2.1	71.8	71.8	71.8	0.0
Т	18,729 <sup>1</sup>	89	644	2.5	71.8	71.8	71.8	0.0
U	19,510 <sup>1</sup>	71	419	2.9	72.0	72.0	72.0	0.0
V	20,382 <sup>1</sup>	91	543	2.1	72.8	72.8	72.8	0.0
W	21,539 <sup>1</sup>	83	464	2.2	73.3	73.3	73.3	0.0
Х	$22,707^{1}$	84	427	1.8	74.3	74.3	74.3	0.0
Y	23,497 <sup>1</sup>	67	358	1.9	74.5	74.5	74.5	0.0
Z	24,833 <sup>1</sup>	64	326	1.7	74.8	74.8	74.8	0.0
Willow Fork Buffalo Bayou								
А	$4.60^{2}$	193	2,278	5.1	105.0	105.0	105.4	0.4
В	$4.80^{2}$	195	2,330	5.0	105.6	105.6	105.8	0.2
С	$5.02^{2}$	175	2,024	5.8	106.6	106.6	106.8	0.2
D	5.28 <sup>2</sup>	158	1,874	6.2	109.4	109.4	109.5	0.1
Ε	5.61 <sup>2</sup>	161	1,953	6.0	110.3	110.3	110.4	0.1
F	$5.90^{2}$	156	1,856	6.3	111.6	111.6	111.6	0.0
G	6.14 <sup>2</sup>	165	1,756	6.7	112.6	112.6	112.6	0.0
Н	6.46 <sup>2</sup>	129	1,681	6.6	114.0	114.0	114.0	0.0
Ι	6.87 <sup>2</sup>	193	2,158	5.1	116.2	116.2	116.3	0.1
J	7.19 <sup>2</sup>	197	1,401	7.9	117.4	117.4	117.6	0.2
К	$7.42^{2}$	650	2,101	5.2	120.6	120.6	121.1	0.5

<sup>1</sup> Feet above confluence with Oyster Creek

<sup>2</sup> Miles above county boundary

TABLE 10

# FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS AND INCORPORATED AREAS

## FLOODWAY DATA

STAFFORD RUN - WILLOW FORK BUFFALO BAYOU

CROSS SECTION	<b>DISTANCE</b> <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD)	WITHOUT FLOODWAY (FEET NAVD)	WITH FLOODWAY (FEET NAVD)	INCREASE (FEET)
Willow Fork								
Buffalo Bayou (continued)								
L	8.14	1,932	11,346	1.0	124.4	124.4	125.4	0.9
М	8.59	2,300	9,448	1.1	125.3	125.3	126.2	0.9
Ν	8.97	2,350	11,352	0.9	126.0	126.0	126.8	0.8
0	9.11	2,200	7,971	1.3	126.2	126.2	127.1	0.9
Р	9.23	1,800	7,621	1.4	127.0	127.0	127.8	0.8
Q	9.42	1,444	6,075	1.7	128.5	128.5	129.1	0.6
R	9.68	1,203	5,415	1.9	129.8	129.8	130.5	0.7
S	10.11	1,148	6,730	1.5	131.4	131.4	132.1	0.7
Т	10.58	673 <sup>2</sup>	2,850	2.5	132.4	132.4	133.1	0.7
U	10.89	1,173	6,435	1.2	132.7	132.7	133.6	0.9
V	11.12	504	1,571	5.2	133.2	133.2	133.9	0.7
W	11.13	505	2,665	3.3	133.9	133.9	134.8	1.0
Х	11.25	335	719	7.9	134.5	134.5	134.8	0.3
Y	11.56	708	2,735	7.3	138.5	138.5	139.2	0.7
Z	11.88	1,400	3,710	1.0	139.0	139.0	139.7	0.7
AA	12.24	662	1,478	2.3	139.8	139.8	140.3	0.5
AB	12.77	482	1,382	2.4	141.6	141.6	142.0	0.4
AC	12.99	738	2,401	1.4	142.0	142.0	142.6	0.6
AD	13.28	87	648	3.4	142.2	142.2	142.9	0.7
AE	13.48	456	1,134	2.0	143.0	143.0	143.5	0.5
AF	13.75	290	348	6.4	144.6	144.6	144.7	0.1
AG	13.93	2,000	1,696	1.3	146.6	146.6	146.9	0.3
AH	14.10	947 <sup>3</sup>	1,128	2.0	147.1	147.1	147.5	0.4

<sup>3</sup> This width extends beyond county boundary

TABLE 10

FEDERAL EMERGENCY MANAGEMENT AGENCY FORT BEND COUNTY, TEXAS FLOODWAY DATA

AND INCORPORATED AREAS

WILLOW FORK BUFFALO BAYOU

#### 5.0 **INSURANCE APPLICATION**

For flood insurance rating purposes, flood insurance zone designations are assigned to a community based on the results of the engineering analyses. These zones are as follows:

Zone A

Zone A is the flood insurance rate zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by approximate methods. Because detailed hydraulic analyses are not performed for such areas, no BFEs or depths are shown within this zone.

Zone AE

Zone AE is the flood insurance rate zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by detailed methods. Whole-foot BFEs derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone AO

Zone AO is the flood insurance rate zone that corresponds to the areas of 1-percent shallow flooding (usually sheet flow on sloping terrain) where average depths are between 1 foot and 3 feet. Average whole-foot depths derived from the detailed hydraulic analyses are shown within this zone.

Zone X

Zone X is the flood insurance rate zone that corresponds to areas outside the 0.2-percent-annualchance floodplain, areas within the 0.2-percent-annual-chance floodplain, areas of 1-percentannual-chance flooding where average depths are less than 1 foot, areas of 1-percent-annualchance flooding where the contributing drainage area is less than 1 sq. mi., and areas protected from the 1.0-percent flood by levees. No BFEs or depths are shown within this zone.

#### 6.0 FLOOD INSURANCE RATE MAP

The FIRM is designed for flood insurance and floodplain management applications.

For flood insurance applications, the map designates flood insurance rate zones as described in Section 5.0 and, in the 1-percent-annual-chance floodplains that were studied by detailed methods, shows selected whole-foot BFEs or average depths. Insurance agents use the zones and BFEs in conjunction with information on structures and their contents to assign premium rates for flood insurance policies. For floodplain management applications, the map shows by tints, screens, and symbols, the 1-percent and 0.2-percent-annual-chance floodplains. Floodways and the locations of selected cross sections used in the hydraulic analyses and floodway computations are shown where applicable.

The current FIRM presents flooding information for the entire geographic area of Fort Bend County. Previously, FIRMs were prepared for each incorporated community and the unincorporated areas of the County identified as flood-prone. Historical data relating to the maps prepared for each community are presented in Table 11, "Community Map History."

	COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISION DATE	FLOOD INSURANCE RATE MAP EFFECTIVE DATE	FLOOD INSURANCE RATE MAP REVISION DATE
	FIRST COLONY L.I.D. <sup>2</sup>	July 9, 1976	December 20, 1977	November 19, 1987	
	FORT BEND COUNTY L.I.D. NO. 2	March 17, 1981	None	February 15, 1985	
	FORT BEND COUNTY L.I.D. NO. 7 <sup>1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	May 3, 1990
	FORT BEND COUNTY M.U.D NO. 2	March 11, 1977	None	November 15, 1984	
	FORT BEND COUNTY M.U.D NO. 25	July 19, 1976	December 20, 1977	February 4, 1987	
	FORT BEND COUNTY M.U.D NO. 34 <sup>1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990
	FORT BEND COUNTY M.U.D NO. 35 <sup>-1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990
	FORT BEND COUNTY M.U.D NO. 41 <sup>1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990
	FORT BEND COUNTY M.U.D NO. 42 <sup>3</sup>	January 17, 1975	October 25, 1977	January 6, 1982	December 17, 1987
	FORT BEND COUNTY M.U.D. NO. 23	June 11, 1985	None	August 5, 1986	
	<sup>1</sup> Dates for this community are those				
	<sup>2</sup> The area of this community was previousl			t identified as a separate NFIP communit	y until November 19, 1987
	Therefore, the earlier dates for this comm <sup>3</sup> The area of this community was previousl			ad as a congrate NEID community	pril 2 2014
	Therefore, the earlier dates for this comm			ieu as a separate INFIP community until A	
Т	FEDERAL EMERGENCY MA		•		
TABLE 11	FORT BEND CO AND INCORPORA		(	COMMUNITY MAP HISTO	DRY

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISION DATE	FLOOD INSURANCE RATE MAP EFFECTIVE DATE	FLOOD INSURANCE RATH MAP REVISION DATE
ARCOLA, CITY OF <sup>1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990
FAIRCHILDS, VILLAGE OF <sup>1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990
FULSHEAR, CITY OF <sup>2</sup>	July 9, 1976	December 20, 1977	August 19, 1987	
KENDLETON, CITY OF	April 29, 1980	None	September 30, 1992	
KINGSBRIDGE M.U.D. <sup>1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	
MISSOURI CITY, CITY OF	January 17, 1975	October 25, 1977	January 6, 1982	December 17, 1987
NEEDVILLE, CITY OF <sup>3</sup>	July 9, 1976	December 20, 1977	March 4, 1987	
PECAN GROVE M.U.D.	November 1, 1977	May 1, 1979	August 4, 1987	

<sup>1</sup> Dates for this community are those of the Fort Bend County Unincorporated Areas

<sup>2</sup> The area of this community was previously shown on the FIRM as unincorporated areas of Fort Bend County and was not identified as a separate NFIP community until August 19, 1987 Therefore, the earlier dates for this community are those of the unincorporated areas of Fort Bend County.

<sup>3</sup> The area of this community was previously shown on the FIRM as unincorporated areas of Fort Bend County and was not identified as a separate NFIP community until March 4, 1987 Therefore, the earlier dates for this community are those of the unincorporated areas of Fort Bend County.

FEDERAL EMERGENCY MANAGEMENT AGENCY

FORT BEND COUNTY, TX AND INCORPORATED AREAS

TABLE 11

## **COMMUNITY MAP HISTORY**

	COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISION DATE	FLOOD INSURANCE RATE MAP EFFECTIVE DATE	FLOOD INSURANCE RATE MAP REVISION DATE		
	BEASLEY, CITY OF <sup>1&amp;2</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990		
	PLEAK, VILLAGE OF <sup>1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990		
	RICHMOND, CITY OF	June 28, 1974	August 22, 1975	March 1, 1982	August 4, 1987		
	ROSENBERG, CITY OF	June 28, 1974	August 22, 1975	December 4, 1984	May 17, 1990		
	SIMONTON, CITY OF	August 4, 1987	None	August 4, 1987			
	STAFFORD, CITY OF	March 1, 1982	None	March 1, 1982			
	SUGAR LAND, CITY OF	May 31, 1974	August 22, 1975	November 4, 1981	October 16, 1987		
	THOMPSONS, TOWN OF <sup>1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990		
	WEST KEEGANS BAYOU IMPROVEMENT DISTRICT <sup>1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990		
	WILLOW FORK DRAINAGE DISTRICT <sup>1</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990		
	Dates for this community are those of the Fort Bend County Unincorporated Areas No Special Flood Hazard Areas identified within Fort Bend County						
TA	FEDERAL EMERGENCY MANAGEMENT AGENCY						
TABLE 11	FORT BEND COUNTY, TX AND INCORPORATED AREAS		COMMUNITY MAP HISTORY				

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISION DATE	FLOOD INSURANCE RATE MAP EFFECTIVE DATE	FLOOD INSURANCE RATE MAP REVISION DATE		
UNINCORPORATED AREAS	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990		
BIG OAKS M.U.D. <sup>1</sup>	November 29, 1985	None	August 5, 1986			
CHELFORD CITY M.U.D. <sup>1</sup>	January 3, 1997	None	January 3, 1997			
MISSION BEND M.U.D. NO. 1 <sup>1, 2</sup>	July 9, 1976	December 20, 1977	August 5, 1986	September 4, 1987		
FORT BEND COUNTY M.U.D. NO. 30 <sup>1</sup>	July 9, 1976	None	January 3, 1997			
MEADOWS PLACE, CITY OF <sup>1</sup>	September 30, 1992	None	September 30, 1992			
ORCHARD, CITY OF <sup>1 &amp; 2</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990		
HOUSTON, CITY OF $^2$	December 27, 1974	April 8, 1977	December 31, 1974	December 11, 1979 September 21, 1982 September 27, 1985 September 4, 1987		
KATY, CITY OF $^2$	June 28, 1974	July 9, 1976 January 24, 1978	March 2, 1981	February 8, 1983		
PEARLAND, CITY OF <sup>2</sup>	January 31, 1975	August 13, 1976	July 5, 1984			
WESTON LAKES, CITY OF <sup>2</sup>	July 9, 1976	December 20, 1977	August 5, 1986	June 3, 1988 May 3, 1990 June 18, 1990		
Dates for this community are those of the Fort Bend County Unincorporated Areas						
FORT BEND COUNTY, TX		COMMUNITY MAP HISTORY				
	UNINCORPORATED AREAS BIG OAKS M.U.D. <sup>1</sup> CHELFORD CITY M.U.D. <sup>1</sup> MISSION BEND M.U.D. NO. 1 <sup>1,2</sup> FORT BEND COUNTY M.U.D. NO. 30 <sup>1</sup> MEADOWS PLACE, CITY OF <sup>1</sup> ORCHARD, CITY OF <sup>1 &amp; 2</sup> HOUSTON, CITY OF <sup>2</sup> KATY, CITY OF <sup>2</sup> PEARLAND, CITY OF <sup>2</sup> WESTON LAKES, CITY OF <sup>2</sup> <sup>1</sup> No Special Flood Hazard Areas ident <sup>2</sup> Dates for this community are thos FEDERAL EMERGENCY MA	UNINCORPORATED AREAS       July 9, 1976         BIG OAKS M.U.D. <sup>1</sup> November 29, 1985         CHELFORD CITY M.U.D. <sup>1</sup> January 3, 1997         MISSION BEND       July 9, 1976         MU.D. NO. 1 <sup>1,2</sup> July 9, 1976         FORT BEND COUNTY       July 9, 1976         MEADOWS PLACE, CITY OF <sup>1</sup> September 30, 1992         ORCHARD, CITY OF <sup>1 &amp; 2</sup> July 9, 1976         HOUSTON, CITY OF <sup>2</sup> July 9, 1976         HOUSTON, CITY OF <sup>2</sup> December 27, 1974         KATY, CITY OF <sup>2</sup> June 28, 1974         PEARLAND, CITY OF <sup>2</sup> July 9, 1976         'No Special Flood Hazard Areas identified within Fort Bend County Unin         'No Special Flood Hazard Areas identified within Fort Bend County Unin         'No Special Flood Hazard Areas identified within Fort Bend County Unin         'PEDERAL EMERGENCY MAJOREMENT AGENCY	COMMUNITY NAME     INITIAL IDENTIFICATION     MAP REVISION DATE       UNINCORPORATED AREAS     July 9, 1976     December 20, 1977       BIG OAKS M.U.D. <sup>1</sup> November 29, 1985     None       CHELFORD CITY M.U.D. <sup>1</sup> January 3, 1997     None       MISSION BEND MU.D. NO. 1 <sup>1,2</sup> July 9, 1976     December 20, 1977       FORT BEND COUNTY M.U.D. NO. 30 <sup>1</sup> July 9, 1976     None       MEADOWS PLACE, CITY OF <sup>1</sup> September 30, 1992     None       ORCHARD, CITY OF <sup>1 &amp; 2</sup> July 9, 1976     December 20, 1977       HOUSTON, CITY OF <sup>2</sup> December 27, 1974     April 8, 1977       KATY, CITY OF <sup>2</sup> June 28, 1974     July 9, 1976       PEARLAND, CITY OF <sup>2</sup> June 28, 1974     July 9, 1976       WESTON LAKES, CITY OF <sup>2</sup> July 9, 1976     December 20, 1977 <sup>1</sup> NO Special Flood Hazard Areas identified within Fort Bend County     Lemember 20, 1977 <sup>1</sup> NO Special Flood Hazard Areas identified within Fort Bend County Unincorporated Areas     FEDERAL EMERGENCY MANAGEMENT AGENCY       FEDERAL EMERGENCY MANAGEMENT AGENCY     FEDERAL EMERGENCY MANAGEMENT AGENCY	COMMUNITY NAME     INITIAL IDENTIFICATION     MAP REVISION DATE     MAP EFFECTIVE DATE       UNINCORPORATED AREAS     July 9, 1976     December 20, 1977     August 5, 1986       BIG OAKS MUD. <sup>1</sup> November 29, 1985     None     August 5, 1986       CHELFORD CITY M.U.D. <sup>1</sup> January 3, 1997     None     January 3, 1997       MISSION BEND MUD. NO. 1 <sup>1,2</sup> July 9, 1976     December 20, 1977     August 5, 1986       PORT BEND COUNTY M.U.D. NO. 30 <sup>1</sup> July 9, 1976     None     January 3, 1997       MEADOWS PLACE, CITY OF <sup>1</sup> September 30, 1992     None     September 30, 1992       ORCHARD, CITY OF <sup>2</sup> July 9, 1976     December 20, 1977     August 5, 1986       HOUSTON, CITY OF <sup>2</sup> July 9, 1976     December 20, 1977     August 5, 1986       HOUSTON, CITY OF <sup>2</sup> July 9, 1976     December 20, 1977     August 5, 1986       HOUSTON, CITY OF <sup>2</sup> July 9, 1976     December 20, 1977     March 2, 1981       PEARLAND, CITY OF <sup>2</sup> June 28, 1974     July 9, 1976     March 2, 1981       WESTON LAKES, CITY OF <sup>2</sup> July 9, 1976     December 20, 1977     August 5, 1986       'No Special Flood Hazard Areas identified within Fort Bend County     'December 20, 1977     August 5, 1986       'No Special Flood Hazard Areas identified within Fort Bend County Unincorporated Areas		

## 7.0 OTHER STUDIES

There are no other known studies underway in Fort Bend County. There are ongoing and completed Flood Insurance Studies in adjacent counties: Harris, Waller, Austin, Brazoria, and Wharton. This FIS report is in agreement with FIS data from those counties. This report either supersedes or is compatible with all previous studies published on streams studied in this report and should be considered authoritative for the purposes of the NFIP.

## 8.0 LOCATION OF DATA

Information concerning the pertinent data used in the preparation of this study can be obtained by contacting FEMA Region VI, Federal Insurance and Mitigation Division, 800 North Loop 288, Denton, Texas 76209.

#### 9.0 <u>BIBLIOGRAPHY AND REFERENCES</u>

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